



**4700 Goshen Rd,
Fort Wayne IN, 46803
Big C Lumber
Project No. 25023**

ADDENDUM NO. 1

June 05, 2026

The following items are revisions, clarifications and additions to the original drawings and project manual for the above referenced project.

Clarification for bidding contractors:

1. Contractors should be including pricing for the main building and the entire site development, inclusive of material pricing from Big C Lumber.
2. Contractors are NOT including pricing to supply and erect the sheds #1 - 8. These will be addressed by change order after completion of the contract. Sheds #1, #2/3 & #7 will require foundations with the sheds being provided from Krauter. Shed #4-6 & #8 will be pole building type buildings.
3. Big C Lumber will be providing pricing for INCLUSION in your bid number for the following items:
 - a. All wood framing materials
 - b. All steel and other exterior siding
 - c. All exterior windows
 - d. All shingles/felt/underlayment/drip edge etc. for roof.
 - e. The 3 sheds, with the exception of the drive through wood shed in the back have been quoted as a steel building through Krauter out of Indy, will provide information upon request.
 - f. All interior doors, trim, cabinets
 - g. Possibly the suspended ceiling tile and gridwork
 - h. LVT flooring for display area and all offices. We may want an option for either polished concrete or epoxy or similar flooring for entrance through the gondola areas.
 - i. All drywall and accessories
 - j. Exterior Signage
 - k. All displays

CIVIL

Sheets C1.0, C2.0 etc.

1. Structure 2A invert elevation south to be 861.79,
2. All hard-surface elevations to be raised approximately 0.4', including top of rims, pavement, FFE, etc.,

VINTAGE ARCHONICS, INC.

8205 Lima Road Fort Wayne, Indiana 46818 (260) 489-3543 (260) 489-1293 fax www.vintagearchonics.com



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3. Invert elevations of pipe, basin elevations, 100-year overflow path out of basin, and other elements and elevations of stormwater system (excluding rim elevations) to remain as shown on design,
4. Additional mounding of topsoil on southeast corner, northern edge, northwest corner and the southwest corner will take place,
5. The drawing showing these changes will be issued Monday morning, June 8, 2026.

STRUCTURAL

Sheet S.101

1. Adjust dock length to match documentation on S.101.
2. Add railing to dock edge per the provided detail.

ARCHITECTURAL

Sheet A.110

1. Include "A+A" callout for the west wall of room 103.

Sheet A.501

1. All sheathing to be 3/4"
2. Section 1/A.501 - D06.006 3/4" sheathing is required at the location of the stone wainscot for backing.

ELECTRICAL

Sheet EL.210

1. Revised Lighting Fixture Schedule for light fixtures W1 and W3.



GME[®]

GME TESTING

A  **UES Company**

Geotechnical Report

GME Project No. G25-093150

Proposed New Lumberyard Facility

Just North of 4632 Goshen Road
Fort Wayne, IN

October 6, 2025

Prepared For:

Big C Lumber
50860 Princess Way
Granger, IN 46530
Attn: Bill Wallace, President/CEO
Copy: Lucas M. Konger, R.A., AIA, ACHA,
President – Vintage Archonics

Prepared By:

GME Testing
3517 Focus Dr
Fort Wayne, IN 46818





October 6, 2025
G25-093150

Big C Lumber

50860 Princess Way
Granger, IN 46530

Attn: Bill Wallace, President/CEO

Copy: Lucas M. Konger, R.A., AIA, ACHA, President – Vintage Archonics

REF: SUBSURFACE EXPLORATION AND RECOMMENDATIONS

Proposed New Lumberyard Facility

Just North of 4632 Goshen Road
Fort Wayne, IN

Gentlemen:

In compliance with your recent request, GME Testing is pleased to submit this report containing the results of our subsurface exploration and geotechnical recommendations for the proposed new lumberyard facility at the above-referenced project. Our services were performed according to our proposal (GMEP 25-080459.01) dated September 12, 2025, and authorized by Mr. Bill Wallace on September 15, 2025.

1.0 INTRODUCTION

1.1 Site Location

Big C Lumber, in collaboration with Vintage Archonics and Abonmarche Group are considering the design and development of a new lumberyard facility on a vacant lot located just North of 4632 Goshen Road in Fort Wayne, Indiana.

1.2 Purpose of Work

The purpose of our geotechnical investigation was to evaluate the subsurface conditions and to develop geotechnical recommendations for use by the owner's architects and design engineers in preparing the design plans for the proposed construction.

Fifteen (15) vertical soil test borings were requested and drilled at the locations provided in this report to evaluate subsurface conditions at the site. Table 1 summarizes the designated boring areas.

Table 1: Summary of Boring Locations			
Proposed Construction	Boring Number	Latitude	Longitude
Parking and Driveway Area	B-01	41.137425	-85.203993
Retail/Storage Building	B-02	41.137752	-85.203893
	B-03	41.137486	-85.203576
	B-04	41.137745	-85.203559
	B-05	41.137891	-85.203345
Storm Detention Area	B-06	41.137881	-85.202985
Parking and Driveway Area	B-07	41.138106	-85.20365
Sheds 7, 8, & 9	B-08	41.138386	-85.204172
	B-09	41.138772	-85.204178
Sheds 4, 5, & 6	B-10	41.138381	-85.203784
	B-11	41.138769	-85.203782
Sheds 2 & 3	B-12	41.138389	-85.203306
	B-13	41.13877	-85.203306
Shed 1	B-14	41.138389	-85.202959
	B-15	41.138769	-85.202963

1.3 Field Coordination

GME Testing coordinated their fieldwork logistics, site access, utilities marking, and the geotechnical drilling program schedule with a client representative to conduct this geotechnical engineering investigation.

1.4 Site Characteristics

1.4.1 Site Features

At the time of our field investigation, the site was a densely vegetated and wooded parcel. Some clearing of trees occurred previously. The densely wooded areas were present along the West, North, and East perimeters of the site. To gain access to the test borings, additional site clearing by GME Testing personnel was necessary to perform our field investigation. Figure 1 shows photographs of the site at the time of subsurface investigation.



Figure 1 – Selected Photographs of the Site

The existing surface elevations were estimated from available Allen County GIS maps and are approximately between El. 862 to El. 870, as shown on the bore logs included in Appendix B for reference only.

It is our understanding that Abonmarche Group will be performing the civil engineering plans and will confirm the existing site surface elevations. GME Testing should be allowed to review the final civil plans. If significant fluctuation from our estimated site elevations are identified, revisions to this report may be necessary.

1.4.2 Site Geology

According to the USDA Web Soil Survey, the site's near-surface soils are primarily composed of Blount loam and Pewamo silty clay loam. A copy of the *Custom Soil Resource Report for Allen County, Indiana*, is included in Appendix B of this report.

1.5 Project Details

Based on the available information and conceptual boring plan (Figure 2), the proposed project will consist of a single-story slab-on-grade retail/storage building, various storage sheds, parking and driveway areas, and a storm detention area.

The retail/storage building and sheds are anticipated to be pre-engineered metal construction and will have cantilever racks. The storage portion of the building may contain a mezzanine area. It is anticipated that the storage areas will be subject to forklift traffic.



A design assumption is that the interior floor slabs will be "floating," indicating that they will be isolated from the footings, except in areas intentionally thickened to support interior load-bearing elements. It is anticipated that the rack storage areas will include reinforcing steel concrete floor slabs placed over adequate stone thickness and a suitably prepared subgrade following the recommendations of this report. The structural engineer will design and detail the building slabs (i.e., thickness design, jointing, reinforcement).

The new parking and driveway areas are expected to include both light-duty and heavy-duty pavement sections. We have provided recommendations for subgrade preparation; however, the design and detailing of the pavement are beyond the scope of our services and will be provided by the civil engineer designer.

The grading plans and the proposed building's finished floor elevations were being prepared and were not available at this writing. It is anticipated that some cutting and filling may be required to achieve the desired grade elevations within the proposed structures and parking areas. Based on estimated site elevations, approximately 2 to 4 feet of cutting and filling may be required within the proposed construction areas. Once the site grading and finished floor elevation are finalized, we should be allowed to review and evaluate whether any changes to this report will be necessary.

GME Testing should be contacted to review design information that conflicts with our stated understanding presented in this report. Based on our review of this information, it will be determined if any changes to this report or our recommendations will be required.

2.0 FIELD INVESTIGATION

2.1 Boring Locations

Our field exploration program consisted of drilling fifteen (15) vertical soil test borings to depths of approximately 10 to 25 feet below the existing ground surface (bgs), as shown on the borehole logs that were requested. Boring B-06 was offset due to existing large tree logs and site features. The boring locations presented in Exhibit A and included in Appendix A of this report are approximate.

Our subsurface exploration was performed in accordance with the Standard Penetration Test, ASTM D-1586. The stratification of soils, as shown on the accompanying boring logs, included in Appendix B of this report, represents the soil conditions at the drilled borehole location. All samples were classified in general accordance with ASTM D-2487. Our test results are included in the individual boring logs included in Appendix B.

2.2 Laboratory Testing Program

Soil samples collected using a split spoon sampler were analyzed in the laboratory following ASTM standard specifications. Our laboratory testing program included performing **a)** visual soil classifications according to ASTM D-2487 and ASTM D-2488, **b)** natural moisture content tests according to ASTM D-2216 on all samples, and **c)** unconfined compressive strength tests and calibrated spring hand penetration tests in general accordance with ASTM D-2166.

2.3 Boring Results

The following summary outlines the materials encountered during the test boring program. Detailed descriptions and data for each test boring are available in the Borehole Logs in Appendix B.

This overview reflects the Site's geotechnical conditions based on our field investigation and laboratory testing. Conditions not represented at boring locations may be encountered during construction and could affect the project.

Surficial Materials: The test borings disclosed varying topsoil thicknesses between approximately 4 to 10 inches. The actual stripping depth of topsoil will vary from the surficial topsoil thickness and can be documented, if needed, by performing test pits observed by GME Testing and/or at the time of earthmoving activities.

Native Soils: The native soils consisted primarily of cohesive soils of very stiff and hard sandy silty clays underlain at about 3 to 6 feet by silty clays changing to gray silty clays below a depth of about 10 to 15 feet through the explored depths of the test borings. Boring B-08 disclosed dense, fine to medium coarse sand at and below a depth of about 13 feet bgs.

In borings B-01 and B-10, the existing clays exhibited medium stiff to stiff consistencies with generally elevated moisture values of 20 percent or more. Drying and aerating similar soils on site will likely be necessary, depending on weather conditions at the time of construction.

The **General Notes** sheet, which follows the boring logs included in Appendix B, provides an explanation of the soil consistency and relative density correlations.

Groundwater Measurements: No groundwater was encountered in the borings during and immediately after our drilling operation. Short-term groundwater measurements were taken during our field operations by noting the depth of water on the rods and in open boreholes following the withdrawal of the drilling augers after the completion of drilling activities in test borings.

The groundwater depths shown on the boring logs reflect groundwater levels **only** for the date on which the borings were drilled. Fluctuations in groundwater levels and the rate of seepage may occur due to variations in rainfall, water levels, and other factors.

3.0 GEOTECHNICAL RECOMMENDATIONS

The following geotechnical recommendations are based on our field and laboratory testing, as well as our evaluation of the subsurface materials encountered in the test borings. Any significant changes in building configuration, location, loading, or assumed grades should be brought to our attention for further evaluation, as they may affect our conclusions.

3.1 General Earthwork and Construction Recommendations

3.1.1 Site Preparation Recommendations

The initial site preparation should include the clearing and removal of overgrown vegetation, tree stumps, root mass, topsoil, and unsuitable soils wherever encountered within the influence of proposed construction areas. A GME Testing engineering technician should be present to observe and document these activities.

After removal and rough grading, GME Testing should be retained to inspect the exposed subgrade areas. This inspection should include at least probing and proof rolling to identify and remediate any inconsistent subgrade conditions, as weather conditions may dictate. This inspection will help determine if any pockets of unstable materials exist beneath the surface.

The near-surface clay soils similar to those observed to be very moist (i.e., B-1 and B-10) will likely require remediation. Remediation of any unsuitable areas will depend on the field conditions at the time of proposed grading, which may consist of aerating, conditioning, followed by recompact, and/or removal and replacement with approved, compacted engineered fill. Any identified issues should be addressed before placing new fill, floor slabs, pavement, or flatwork.

Earthwork operations should be scheduled during warm, dry conditions whenever possible to reduce subgrade instability and delays. Construction traffic should be restricted to designated access routes to prevent pumping, yielding, or localized subgrade failures from repeated loading over prepared areas.

3.1.2 Engineered Fill

All engineered fill needed to replace undercut materials or as a grade-raise fill should be approved by GME Testing before placement on site. The existing very stiff to hard sandy and silty clays encountered in moist test borings can be used as grade-raise fill, provided that samples of such material are collected for

laboratory testing and evaluation. All fill materials shall be compacted to achieve a dry density of 95 percent according to ASTM D-1557.

To achieve the recommended compaction limit of the structural fill, the fill material should be placed and compacted in layers not exceeding 8 inches in loose thickness (the loose lift thickness should be reduced to 6 inches when utilizing small hand compactors) and within the range of ± 2 percent (%) of the optimum moisture content value. All fill placements should be monitored by a GME Testing representative.

3.1.3 Site Drainage

Maintaining positive surface drainage away from the building is essential to prevent water infiltration and damage. Surface water, including downspouts, should be directed to designated outfalls via properly sloped swales. Any water encountered during foundation or pad preparation should be promptly dewatered and directed away from the construction area and existing structures.

3.1.4 Temporary Excavations

We recommend that trenches and excavations be designed and constructed in compliance with OSHA regulations, specifically OSHA 29 CFR Part 1926, Subpart P, which addresses excavation and trench safety. Trenches and excavations for utilities and other construction activities present risks of sidewall collapse, potentially exposing workers to engulfment hazards. As defined by OSHA, a competent person should monitor all excavations, and appropriate shoring or sloping techniques must be employed to prevent cave-ins.

3.2 Buildings

3.2.1 Foundation Recommendations

After successful site preparation and filling as discussed in **Sections 3.1.1** and **3.1.2**, the proposed constructions can be supported on conventional footings, including column (square type) and continuous (strip wall) footings. Such footings can be designed for a maximum net allowable soil bearing pressure of **3,000 pounds per square foot (psf)** for column (square type) and **2,500 psf** for strip (wall type) footings. The provided soil-bearing capacity values must be confirmed by performing additional quality control inspections as recommended in this report.

Provided that our recommendations in this report and project specifications are followed, total foundation settlements are not expected to exceed about (1) inch with differential settlements of up to (1/2) inch. Field control and proper footing proportions will contribute substantially to minimizing total and differential settlements.

GME Testing recommends that the above footings be supported on approved native very stiff, sandy and silty clay soils and/or within well-compacted engineered fill extending from approved native soils. The engineered fill should be compacted to at least 95 percent of the maximum dry density as determined by ASTM D1557, provided it is placed over an approved subgrade.

Soils that are soft, compressible, unstable, or consist of undocumented fill are not acceptable to support foundations and/or settlement-sensitive construction elements.

Footing extended below their conventional depth of excavations due to unsuitable fill or native soils should be re-established to the desired grade using a lean concrete mix (i.e., 2,000 psi compressive strength after 28 days). Exhibit B, included in Appendix A of this report, provides this illustration.

When using net pressure, the weight of the footing and backfill over the footing, including the weight of the floor slab, need not be considered. Hence, only loads applied at or above the finished floor must be used to dimension the footings.

New fill soil below, above, and surrounding footings shall consist of approved materials and then be placed and compacted in accordance with this report.

If any water infiltration is encountered in the footing excavations, it should be removed by adequate sumps placed outside the limits of the main footing excavations.

Lateral forces on the shallow spread footing are resisted by passive earth pressure and friction between the footing base and subgrade, with an allowable coefficient of friction of 0.35 applied to the minimum downward load. The allowable passive pressure against the side of the foundation for that portion below a depth of 3.5 feet below the exterior grade should not exceed 400 psf per foot of depth. No portion of the soil above this depth should be used when calculating lateral resistance.

It is very critical that the foundation be sized and reinforced to account for all applicable structural loadings (i.e., vertical downward, uplift, overturning, and lateral loadings).

Foundation Frost Depth

All approved foundations resting on competent-bearing soils must be protected during and post-construction. All exterior footings and footings in unheated areas should be located at a depth of 3.5 feet below the final exterior grade for frost protection. Interior footings in heated areas can be located at nominal depths below the finished floor, provided that they bear on firm materials and provided that other recommendations in this report are followed.

Foundation Inspection

It is recommended that GME Testing be retained to perform continuous inspection of all earth-connected phases of the project and be monitored and documented. This is to check that each footing excavation is evaluated during the construction and installation of all foundations and slabs to allow adherence to the recommendations outlined in this report, plans, and project specifications.

To reduce the bearing disturbance caused by inclement weather, sloughing, etc., foundation excavations should be backfilled or concreted immediately following satisfactory evaluation by GME Testing. If this is not practical, the foundation excavation should be adequately protected. Soils exposed at the bases of all excavations must be protected against any detrimental change in conditions, such as from disturbance, rain, and freezing.

Seismicity Classification

A seismic site class "D" may be applied for this project, with a seismic report included in Appendix B. The design parameters presented in Table 2 are based on this classification. The ASCE 7-10 standard utilizes a site profile extending to a depth of 100 feet for seismic site classification. Since the borings for this project did not reach that depth, the site properties below 100 feet were estimated based on our experience and understanding of the geologic conditions in the area.

Table 2: Seismicity Classification Parameters	
Description	Value
*ASCE 7-10 Site Classification	D
Site Latitude	41.137745
Site Longitude	-85.203559
Risk Category	III
Seismic Design Category	B
S _s	0.113g
S ₁	0.061g

Table 2: Seismicity Classification Parameters	
Description	Value
S _{DS}	0.120
S _{D1}	0.097

* Note: The updated ASCE 7-22 seismic classification parameters can also be available and provided, if needed, upon request.

3.2.2 Ground-Supported Slab Recommendations

The site will require remediation due to the presence of unsuitable fill materials. Our guideline recommendations were discussed previously and further provided in **Sections 3.2.1 and 3.2.2** of this report.

It may be necessary to design the slabs-on-grade using methods prescribed by the Portland Cement Association (PCA) and/or the American Concrete Institute (ACI), depending on the loading and performance requirements.

The upper 6 inches of the slab subbase should consist of an approved crushed limestone aggregate, such as INDOT No. 53, to provide a uniform leveling course and serve as a capillary break between the slab and underlying subgrade soils.

A qualified structural engineer should design the ground-supported slabs to accommodate the anticipated loadings. Joint detailing, reinforcement, and load transfer mechanisms should also be addressed by the structural engineer.

Depending on the selected floor finishes, it may also be appropriate to install a moisture barrier below the slab. This decision should be evaluated by the architect and structural engineer based on the intended floor use, type of finishes, and in accordance with ACI recommendations.

We further recommend that slab-on-grade subgrade soils be protected from freezing during winter construction. If frozen soils are encountered, they must be thawed and recompacted or removed and replaced prior to slab construction.

3.3 Parking and Driveway Subgrade Areas

Final grading plans for the parking and driveway areas were not available at the time of this report. Prior to pavement construction, subgrade preparation should follow the recommendations presented herein, including those in **Sections 3.1.1 and 3.1.2**. The following minimum guidelines are provided.

Pavement subgrade quality and performance depend on factors such as weather conditions, construction traffic, material properties, and compaction effectiveness. To ensure consistent support, all fill beneath pavement areas must meet the criteria outlined in this report. Pavement design, materials, and construction should conform to current INDOT Standards and Specifications.

Based on soil conditions observed in test borings (B-01 and B-07), and assuming proper subgrade preparation, a preliminary California Bearing Ratio (CBR) of approximately 3 percent (resilient modulus equivalent to about 4,500 psi) may be used for design estimates.

Subgrade strength can be significantly reduced during and after wet weather due to increased moisture. To mitigate this, earthwork and site preparation should be scheduled during seasonally dry periods with minimal rainfall. When conditioning the subgrade is impractical due to weather or other factors, replacement with approved materials may be necessary.

Proper surface and subsurface drainage are critical to preventing moisture infiltration that reduces pavement service life. The subgrade should be uniformly sloped to facilitate drainage through the granular base to shoulders or inlets, thereby preventing water ponding beneath the pavement.

3.4 Storm Detention Area

Soils encountered in the proposed stormwater detention basin area (i.e., boring B-06) consisted of cohesive silty and sandy silty clays extending to the terminal depth of the boring.

When recompacted, the on-site clayey soils can be expected to exhibit low rates of seepage, in the range of approximately 1×10^{-6} to 1×10^{-7} cm/sec. In their native, undisturbed condition, however, these soils often contain granular seams, resulting in higher horizontal than vertical permeability. The permeability values provided are approximate and unfactored. Due to subsurface variability, we recommend adopting a conservative approach in both hydraulic and geotechnical design.

To prevent erosion of basin side slopes, grass cover is generally acceptable where root systems can withstand anticipated surface runoff velocities. In concentrated flow paths or steeper areas, the civil engineer should evaluate the need for supplemental erosion protection, such as riprap. Slopes should be periodically monitored for erosion and repaired as needed.

On-Site Borrow Material Source

Depending on their condition at the time of construction, borrow materials such as brown silty clay and sandy silty clay may be reused as general grade-raise fill if properly moisture-conditioned and compacted. GME Testing should be present during excavation to observe soil stratigraphy, perform field classification tests, and confirm material suitability for reuse. Extreme care should be taken to avoid mixing suitable and unsuitable materials, and no borrow fill should be placed without prior approval from the geotechnical engineer.

4.0 LIMITATIONS

This field evaluation, laboratory testing, and geotechnical analyses presented in this geotechnical investigation report have been conducted in accordance with current practice and the standard of care exercised by geotechnical consultants performing similar tasks in the project area. Although individual test borings are representative of the subsurface conditions at the boring locations on the dates drilled, they are not necessarily representative of the subsurface conditions between boring locations or subsurface conditions during other seasons of the year.

The assessment of the site for flooding potential is beyond the scope of our geotechnical analysis. Environmental issues were also beyond our work scope. Such aspects should be evaluated and confirmed by a qualified professional engineer.

The lines of demarcation shown on the logs represent approximate boundaries between the various classifications. The stratification of soils, as shown on the accompanying test borehole logs, represents the soil conditions at the drilled borehole locations, and variations may occur between the boreholes. In-situ strata changes could occur gradually or at different levels. Also, it should be noted that the boreholes depict conditions at the particular locations and times indicated.

The report was prepared by GME Testing solely for the use of the Client in accordance with an executed contract. The Client's use of or reliance on this report is limited by the terms and conditions of the contract and by the qualifications and limitations stated in the report. It is also acknowledged that the Client's use of and reliance of this report is limited for reasons that include actual site conditions that may change with time; hidden conditions not discoverable within the scope of the assessment may exist at the site; and the scope of the investigation may have been limited by time, budget and other constraints imposed by the client.

Neither the report nor its contents, conclusions, or recommendations are intended for the use of any party other than the Client. GME Testing and the Client assume no liability for any reliance on this report by such party. The rights of the client under contract may not be assigned to any person or entity, without the consent of GME Testing which shall not be unreasonably withheld.

Our services have been provided consistent with its professional standards of care. No other warranties are made, either expressed or implied.

We wish to remind you that we will store the samples for 30 days, after which time they will be discarded unless you request otherwise.

We appreciate the opportunity to be of service on this project. Should you have any questions related to this report, please contact us at your convenience.

Sincerely,
GME Testing

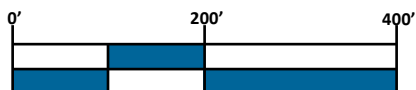


Rami M. Anabtawi, P.E., BC.GE



Moe Kyaw, E.I.

APPENDIX A



GRAPHIC SCALE: 1" = 200'

LEGEND



APPROXIMATE BORING LOCATION

NOTE:

1. AERIAL IMAGE TAKEN FROM GOOGLE EARTH
WITH AN IMAGE DATE OF 07/03/2022.



EXHIBIT A

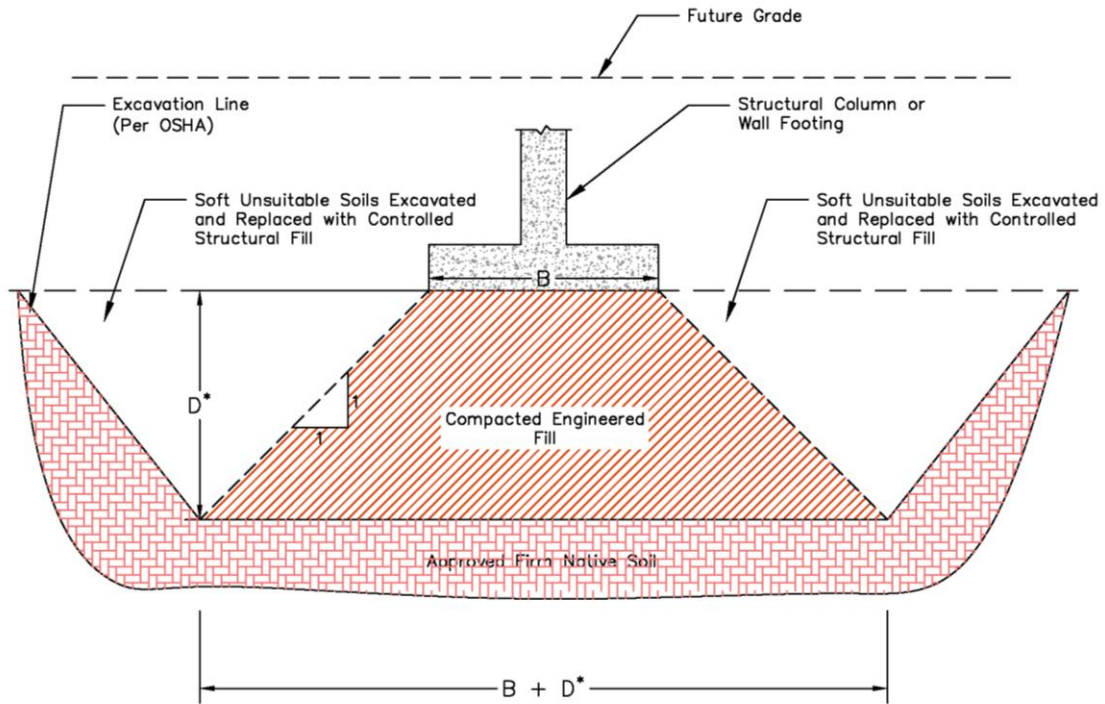
BORING LOCATION DIAGRAM

Proposed New Lumberyard Facility

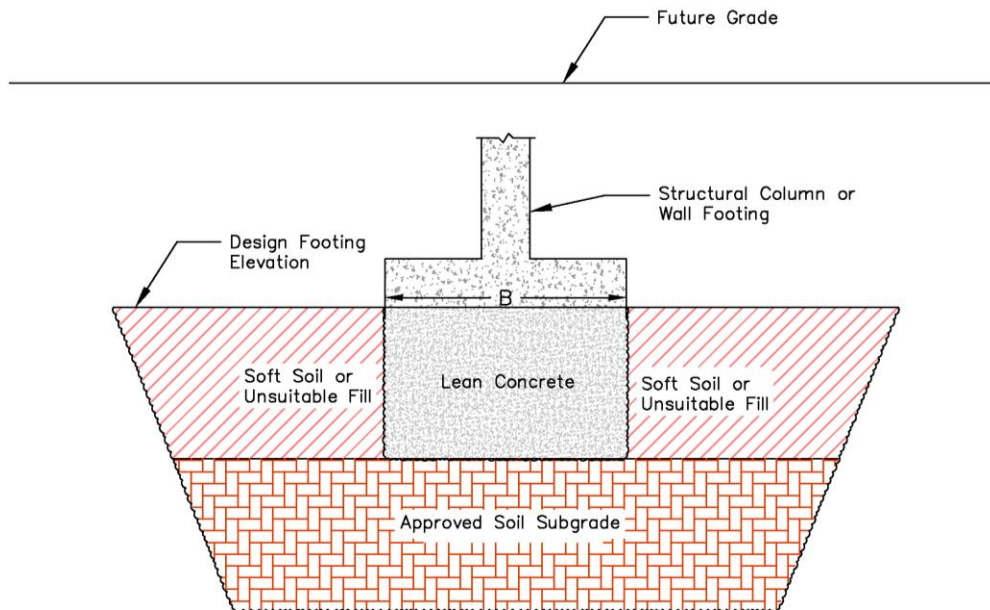
Just North of 4632 Goshen Road, Fort Wayne, IN

No.	Revision	Date
		Date: 10/6/25
		Prepared By: MK
		Scale: AS NOTED
		Client: Big C Lumber
		GME Project No.: G25-093150

UNDERCUT EXCAVATION FOR FOOTINGS IN UNSTABLE MATERIALS REPLACED WITH COMPACTED STRUCTURAL FILL



UNDERCUT EXCAVATION FOR FOOTINGS IN UNSUITABLE MATERIALS REPLACED WITH LEAN CONCRETE



APPENDIX B

TEST BORING LOG

BORING NO.: **B-01**

SHEET **1** OF **1**

GME PROJECT NO: **G25-093150**

STRUCTURE _____

DATUM : _____

DATE STARTED : **09-18-25**

DRILLER/INSP : **RS/RS**

CLIENT: **Big C Lumber**

PROJECT TYPE : **Proposed New Lumberyard Facility**

LOCATION : **Just North of 4632 Goshen Road, Fort Wayne, IN**

ELEVATION : **866.0**

STATION : _____

OFFSET : _____

LINE : _____

DEPTH : **10.0 ft**

BORING METHOD : **ASTM D-1586**

RIG TYPE : **Skid**

CASING DIA. : **3.3 in**

HAMMER : **Auto**

LATITUDE : **41.137425**

LONGITUDE : **-85.203993**

GROUNDWATER: ☒ Encountered at **Dry** ☒ At completion **Dry**

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
865.3		±8" Dark Brown, Silty Sandy Clayey TOPSOIL. 0.7							
	2.5		SS 1	3-4-6 (10)	100	21.0	3.5	3.0	
		Brown and Gray, Moist, SILTY CLAY.							
	5.0		SS 2	2-3-3 (6)	100	23.4	2.9	2.5	
860.0									
	7.5		SS 3	4-6-8 (14)	100	16.2	6.3	4.5+	
		Brown, Moist, SILTY CLAY.							
856.0	10.0		SS 4	7-7-9 (16)	100	15.7	5.9	4.5+	
		Bottom of Boring at 10.0 ft							
	12.5								
	15.0								
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								



TEST BORING LOG

BORING NO.: **B-04**

SHEET **1** OF **1**

GME PROJECT NO: **G25-093150**

STRUCTURE _____

DATUM : _____

DATE STARTED : **09-22-25**

DRILLER/INSP : **JS/AB**

CLIENT: **Big C Lumber**

PROJECT TYPE : **Proposed New Lumberyard Facility**

LOCATION : **Just North of 4632 Goshen Road, Fort Wayne, IN**

ELEVATION : **866.0**

BORING METHOD : **ASTM D-1586**

LATITUDE : **41.137745**

STATION : _____

RIG TYPE : **Skid**

LONGITUDE : **-85.203559**

OFFSET : _____

CASING DIA. : **3.3 in**

LINE : _____

HAMMER : **Auto**

DEPTH : **25.0 ft**

GROUNDWATER: ☒ Encountered at **Dry** ☒ At completion **Dry**

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
865.7		±3.5" Brown, Silty Sandy TOPSOIL. 0.3							
	2.5		SS 1	7-8-12 (20)	100	14.3		4.5+	
	5.0	Brown, SANDY SILTY CLAY, Trace Fine Gravel.	SS 2	8-10-11 (21)	100	13.6		4.5+	
	7.5		SS 3	9-12-14 (26)	100	14.2		4.5+	
857.5									
	10.0		SS 4	12-16-20 (36)	100	16.3		4.5+	
	12.5								
	15.0		SS 5	8-11-16 (27)	100	18.3	7.7	4.5+	
	17.5	Brown, Moist, SILTY CLAY, Trace Fine Sand.							
	20.0		SS 6	7-12-19 (31)	100	13.8	4.5	4.0	
	22.5								
841.0									
	25.0		SS 7	8-16-24 (40)	100	13.3	6.7	4.5+	
		Bottom of Boring at 25.0 ft							
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-06**

SHEET **1** OF **1**

GME PROJECT NO: **G25-093150**

STRUCTURE _____

DATUM : _____

DATE STARTED : **09-22-25**

DRILLER/INSP : **JS/AB**

CLIENT: **Big C Lumber**

PROJECT TYPE : **Proposed New Lumberyard Facility**

LOCATION : **Just North of 4632 Goshen Road, Fort Wayne, IN**

ELEVATION : **860.0**

STATION : _____

OFFSET : _____

LINE : _____

DEPTH : **15.0 ft**

BORING METHOD : **ASTM D-1586**

RIG TYPE : **Skid**

CASING DIA. : **3.3 in**

HAMMER : **Auto**

LATITUDE : **41.137881**

LONGITUDE : **-85.202985**

GROUNDWATER: ☒ Encountered at **Dry** ☒ At completion **Dry**

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
859.7		±4" Brown, Silty Sandy TOPSOIL.							
	2.5		SS 1	7-10-12 (22)	100	15.7		4.5+	
	5.0	Brown, SANDY SILTY CLAY, Trace Fine Gravel	SS 2	7-9-9 (18)	100	14.8	4.7	4.5+	
	7.5	Brown and Gray @ ± 6.5'.	SS 3	8-10-12 (22)	100	15.5		4.5+	
850.0	10.0		SS 4	12-14-18 (32)	100	14.3	4.5	4.5+	
	12.5	Brown, Moist, SANDY SILTY CLAY, Trace Fine Gravel.							
845.0	15.0		SS 5	7-10-10 (20)	100	10.7	6.5	4.5+	
		Bottom of Boring at 15.0 ft							
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-07**
 SHEET 1 OF 1
GME PROJECT NO: **G25-093150**
 STRUCTURE _____
 DATUM : _____
 DATE STARTED : 09-24-25
 DRILLER/INSP : JS/AB

CLIENT: Big C Lumber
 PROJECT TYPE : Proposed New Lumberyard Facility
 LOCATION : Just North of 4632 Goshen Road, Fort Wayne, IN

ELEVATION : <u>864.0</u>	BORING METHOD : <u>ASTM D-1586</u>	LATITUDE : <u>41.138106</u>
STATION : _____	RIG TYPE : <u>Skid</u>	LONGITUDE : <u>-85.20365</u>
OFFSET : _____	CASING DIA. : <u>3.3 in</u>	
LINE : _____	HAMMER : <u>Auto</u>	
DEPTH : <u>10.0 ft</u>		

GROUNDWATER: ☒ Encountered at Dry ☒ At completion Dry

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
863.7		±4" Brown, Silty Sandy TOPSOIL. 0.3							
	2.5		SS 1	6-12-10 (22)	100	15.0		4.5+	
	5.0	Brown, SANDY SILTY CLAY, Trace Fine Gravel.	SS 2	14-18-20 (38)	100	14.3		4.5+	
	7.5		SS 3	15-16-24 (40)	100	15.2		4.5+	
855.5		8.5							
854.0	10.0	Brown, SILTY CLAY. 10.0	SS 4	10-16-20 (36)	100	15.6	9.0	4.5+	
		Bottom of Boring at 10.0 ft							
	12.5								
	15.0								
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-08**
 SHEET 1 OF 1
GME PROJECT NO: **G25-093150**
 STRUCTURE _____
 DATUM : _____
 DATE STARTED : 09-24-25
 DRILLER/INSP : JS/AB

CLIENT: Big C Lumber
 PROJECT TYPE : Proposed New Lumberyard Facility
 LOCATION : Just North of 4632 Goshen Road, Fort Wayne, IN

ELEVATION : <u>864.0</u>	BORING METHOD : <u>ASTM D-1586</u>	LATITUDE : <u>41.138386</u>
STATION : _____	RIG TYPE : <u>Skid</u>	LONGITUDE : <u>-85.204172</u>
OFFSET : _____	CASING DIA. : <u>3.3 in</u>	
LINE : _____	HAMMER : <u>Auto</u>	
DEPTH : <u>15.0 ft</u>		

GROUNDWATER: ☒ Encountered at Dry ☒ At completion Dry

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
863.5		±5.5" Brown, Silty Sandy TOPSOIL. ----- 0.5							
	2.5		SS 1	6-6-7 (13)	100	16.1		4.5+	
	5.0	Brown, SANDY SILTY CLAY, Trace Fine Gravel.	SS 2	8-12-16 (28)	100	14.3		4.5+	
857.0	7.5	----- 7.0	SS 3	9-15-19 (34)	100	14.6		4.5+	
	10.0	Brown, Moist, SILTY CLAY, Trace Fine Gravel.	SS 4	18-24-28 (52)	100	15.2		4.5+	
851.0	12.5	----- 13.0							
849.0	15.0	Brown, Fine to Medium Coarse SAND, Trace Fine Gravel. ----- 15.0	SS 5	18-30-34 (64)	100	7.0			
		Bottom of Boring at 15.0 ft							
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-09**

SHEET **1** OF **1**

GME PROJECT NO: **G25-093150**

STRUCTURE _____

DATUM : _____

DATE STARTED : **09-24-25**

DRILLER/INSP : **JS/AB**

CLIENT: **Big C Lumber**

PROJECT TYPE : **Proposed New Lumberyard Facility**

LOCATION : **Just North of 4632 Goshen Road, Fort Wayne, IN**

ELEVATION : **870.0**

STATION : _____

OFFSET : _____

LINE : _____

DEPTH : **15.0 ft**

BORING METHOD : **ASTM D-1586**

RIG TYPE : **Skid**

CASING DIA. : **3.3 in**

HAMMER : **Auto**

LATITUDE : **41.138772**

LONGITUDE : **-85.204178**

GROUNDWATER: ☒ Encountered at **Dry** ☒ At completion **Dry**

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
869.5		±6" Brown, Silty Sandy TOPSOIL.							
	2.5		SS 1	9-12-15 (27)	100	10.8		4.5+	
	5.0		SS 2	14-18-22 (40)	100	11.8		4.5+	
	7.5	Brown, SANDY SILTY CLAY, Trace Fine Gravel.	SS 3	16-22-27 (49)	100	11.2		4.5+	
	10.0		SS 4	10-18-24 (42)	100	13.0		4.5+	
857.5	12.5								
		Brown, Moist, SILTY CLAY, Trace Fine Gravel.	SS 5	12-20-24 (44)	100	14.8	9.0	4.5+	
855.0	15.0								
		Bottom of Boring at 15.0 ft							
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-10**

SHEET 1 OF 1
GME PROJECT NO: **G25-093150**

STRUCTURE _____

DATUM : _____

DATE STARTED : 09-24-25

DRILLER/INSP : JS/AB

CLIENT: Big C Lumber

PROJECT TYPE : Proposed New Lumberyard Facility

LOCATION : Just North of 4632 Goshen Road, Fort Wayne, IN

ELEVATION : 864.0

BORING METHOD : ASTM D-1586

LATITUDE : 41.138381

STATION : _____

RIG TYPE : Skid

LONGITUDE : -85.203784

OFFSET : _____

CASING DIA. : 3.3 in

LINE : _____

HAMMER : Auto

DEPTH : 15.0 ft

GROUNDWATER: ☒ Encountered at Dry ☒ At completion Dry

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
863.6		±5" Brown, Silty Sandy TOPSOIL.							
	2.5		SS 1	5-6-5 (11)	100	20.3		4.5+	
	5.0	Brown and Gray, SILTY CLAY.	SS 2	4-5-5 (10)	100	20.5		4.5+	
858.0	6.0		SS 3	5-6-7 (13)	100	21.7		4.0	
	10.0	Brown, Moist, SILTY CLAY.	SS 4	6-10-14 (24)	100	15.0	12.7	4.5+	
849.0	15.0		SS 5	7-12-19 (31)	100	16.1		4.5+	
		Bottom of Boring at 15.0 ft							
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-11**

SHEET **1** OF **1**

GME PROJECT NO: **G25-093150**

STRUCTURE _____

DATUM : _____

DATE STARTED : **09-24-25**

DRILLER/INSP : **JS/AB**

CLIENT: **Big C Lumber**

PROJECT TYPE : **Proposed New Lumberyard Facility**

LOCATION : **Just North of 4632 Goshen Road, Fort Wayne, IN**

ELEVATION : **870.0**

STATION : _____

OFFSET : _____

LINE : _____

DEPTH : **15.0 ft**

BORING METHOD : **ASTM D-1586**

RIG TYPE : **Skid**

CASING DIA. : **3.3 in**

HAMMER : **Auto**

LATITUDE : **41.138769**

LONGITUDE : **-85.203782**

GROUNDWATER: ☒ Encountered at **Dry** ☒ At completion **Dry**

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
869.5		±6" Brown, Silty Sandy TOPSOIL.							
	2.5		SS 1	7-9-12 (21)	100	12.8		4.5+	
	5.0	Brown, SANDY SILTY CLAY, Trace Fine Gravel.	SS 2	12-14-15 (29)	100	11.7		4.5+	
862.5	7.5		SS 3	14-15-18 (33)	100	12.0		4.5+	
	10.0		SS 4	7-9-12 (21)	100	15.0	7.3	4.5+	
	12.5	Brown, Moist, SILTY CLAY, Trace Fine Gravel.							
855.0	15.0		SS 5	8-9-14 (23)	100	14.4	12.7	4.5+	
	17.5	Bottom of Boring at 15.0 ft							
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-12**

SHEET **1** OF **1**
GME PROJECT NO: **G25-093150**

STRUCTURE

DATUM :

DATE STARTED : **09-22-25**

DRILLER/INSP : **JS/AB**

CLIENT: **Big C Lumber**

PROJECT TYPE : **Proposed New Lumberyard Facility**

LOCATION : **Just North of 4632 Goshen Road, Fort Wayne, IN**

ELEVATION : **862.0**

BORING METHOD : **ASTM D-1586**

LATITUDE : **41.138389**

STATION :

RIG TYPE : **Skid**

LONGITUDE : **-85.203306**

OFFSET :

CASING DIA. : **3.3 in**

LINE :

HAMMER : **Auto**

DEPTH : **15.0 ft**

GROUNDWATER: ☒ Encountered at **Dry** ☒ At completion **Dry**

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
861.5		±6" Brown, Sandy TOPSOIL.							
	2.5		SS 1	8-10-10 (20)	100	16.6	5.5	4.5+	
	5.0	Brown and Gray, SANDY SILTY CLAY.	SS 2	5-8-9 (17)	100	17.0		4.5+	
856.0	6.0		SS 3	6-7-12 (19)	100	15.9		4.5+	
	10.0	Brown, SILTY CLAY, Trace Fine Gravel.	SS 4	16-18-20 (38)	100	15.4	10.0	4.5+	
847.0	15.0		SS 5	8-9-11 (20)	100	11.0	6.5	4.5+	
		Bottom of Boring at 15.0 ft							
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-13**
 SHEET 1 OF 1
GME PROJECT NO: **G25-093150**
 STRUCTURE _____
 DATUM : _____
 DATE STARTED : 09-24-25
 DRILLER/INSP : JS/AB

CLIENT: Big C Lumber
 PROJECT TYPE : Proposed New Lumberyard Facility
 LOCATION : Just North of 4632 Goshen Road, Fort Wayne, IN

ELEVATION : <u>870.0</u>	BORING METHOD : <u>ASTM D-1586</u>	LATITUDE : <u>41.13877</u>
STATION : _____	RIG TYPE : <u>Skid</u>	LONGITUDE : <u>-85.203306</u>
OFFSET : _____	CASING DIA. : <u>3.3 in</u>	
LINE : _____	HAMMER : <u>Auto</u>	
DEPTH : <u>15.0 ft</u>		

GROUNDWATER: ☒ Encountered at Dry ☒ At completion Dry

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
869.7		±4" Brown, Silty Sandy TOPSOIL. _____ 0.3							
	2.5		SS 1	8-8-10 (18)	100	15.3		4.5+	
	5.0	Brown, SANDY SILTY CLAY.	SS 2	8-12-16 (28)	100	12.1		4.5+	
	7.5		SS 3	9-15-20 (35)	100	12.9		4.5+	
861.5		_____ 8.5							
	10.0		SS 4	10-12-16 (28)	100	15.8	11.4	4.5+	
	12.5	Brown, Moist, SILTY CLAY, Trace Fine Gravel.							
855.0		_____ 15.0							
	15.0		SS 5	12-14-18 (32)	100	15.0	12.7	4.5+	
		Bottom of Boring at 15.0 ft							
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-14**

SHEET 1 OF 1

GME PROJECT NO: **G25-093150**

STRUCTURE _____

DATUM : _____

DATE STARTED : 09-22-25

DRILLER/INSP : JS/AB

CLIENT: Big C Lumber

PROJECT TYPE : Proposed New Lumberyard Facility

LOCATION : Just North of 4632 Goshen Road, Fort Wayne, IN

ELEVATION : 866.0

STATION : _____

OFFSET : _____

LINE : _____

DEPTH : 15.0 ft

BORING METHOD : ASTM D-1586

RIG TYPE : Skid

CASING DIA. : 3.3 in

HAMMER : Auto

LATITUDE : 41.138389

LONGITUDE : -85.202959

GROUNDWATER: ☒ Encountered at Dry ☒ At completion Dry

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
865.6		±4.5" Brown, Silty Sandy TOPSOIL.							
	2.5		SS 1	8-9-9 (18)	100	13.6		4.5+	
	5.0		SS 2	18-28-30 (58)	100	11.4		4.5+	
	7.5		SS 3	19-30-34 (64)	100	12.2		4.5+	
856.0	10.0		SS 4	18-18-22 (40)	100	14.7	7.7	4.5+	
	12.5								
	15.0		SS 5	9-10-10 (20)	100	17.2	4.9	4.5+	
851.0									
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

TEST BORING LOG

BORING NO.: **B-15**

SHEET **1** OF **1**

GME PROJECT NO: **G25-093150**

STRUCTURE

DATUM :

DATE STARTED : **09-22-25**

DRILLER/INSP : **JS/AB**

CLIENT: **Big C Lumber**

PROJECT TYPE : **Proposed New Lumberyard Facility**

LOCATION : **Just North of 4632 Goshen Road, Fort Wayne, IN**

ELEVATION : **868.0**

STATION :

OFFSET :

LINE :

DEPTH : **15.0 ft**

BORING METHOD : **ASTM D-1586**

RIG TYPE : **Skid**

CASING DIA. : **3.3 in**

HAMMER : **Auto**

LATITUDE : **41.138769**

LONGITUDE : **-85.202963**

GROUNDWATER: ☒ Encountered at **Dry** ☒ At completion **Dry**

STRATUM ELEVATION	SAMPLE DEPTH	SOIL/MATERIAL DESCRIPTION	SAMPLE NUMBER	SPT per 6" (N)	% RECOVERY	% MOISTURE CONTENT	UNCONF. COMP., tsf	Qp (tsf)	REMARKS
867.6		±5" Brown, Silty Sandy TOPSOIL.							
	2.5		SS 1	7-10-14 (24)	100	14.5		4.5+	
	5.0	Brown, SANDY SILTY CLAY, Trace Fine Gravel.	SS 2	10-10-16 (26)	100	11.6		3.0	
	7.5		SS 3	10-14-20 (34)	100	11.6			
859.5									
	10.0		SS 4	16-17-20 (37)	100	16.0	10.4	4.5+	
	12.5	Brown, Moist, SILTY CLAY, Trace Fine Gravel.							
853.0									
	15.0		SS 5	10-12-12 (24)	100	15.9	6.7	4.0	
		Bottom of Boring at 15.0 ft							
	17.5								
	20.0								
	22.5								
	25.0								
	27.5								
	30.0								

GENERAL NOTES

SAMPLE IDENTIFICATION

Visual soil classifications are made in general accordance with the United States Soil Classification System on the basis of textural and particle size categorization, and various soil behavior and characteristics. Visual classifications should be made by appropriate laboratory testing when more exact soil identification is required to satisfy specific project applications criteria.

RELATIVE PROPORTIONS OF COHESIONLESS SOILS

<u>Term</u>	<u>Defining Range by % of Weight</u>
Trace	1-10 %
Little	11-20 %
Some	21-35 %
And	36-50 %

WATER LEVEL MEASUREMENT

NE	No Water Encountered
BF	Backfilled upon Completion

ORGANIC CONTENT BY COMBUSTION METHOD

<u>Soil Description</u>	<u>LOI</u>
w/ organic matter	4-15 %
Organic Soil (A-8)	16-30 %
Peat (A-8)	More than 30%

LABORATORY TESTS

Qp	Penetrometer Reading, tsf
Qu	Unconfined Strength, tsf
MC	Moisture Content, %
LL	Liquid Limit, %
PL	Plastic Limit, %
PI	Plastic Index
SL	Shrinkage Limit, %
pH	Measure of Soil Alkalinity/Acidity
γ	Dry Unit Weight, pcf
LOI	Loss of Ignition, %

DRILLING AND SAMPLING SYMBOLS

AS	Auger Sample
BS	Bag Sample
PID	Photo ionization Detector (Hnu meter) volatile vapor level,(PPM)
COA	Clean-Out Auger
CS	Continuous Sampling
FA	Flight Auger
HA	Hand Auger
HAS	Hollow Stem Auger
NR	No Recovery
PT	3" O.D. Piston Tube Sample
RB	Rock Bit
RC	Rock Coring
REC	Recovery
RQD	Rock Quality Designation
RS	Rock Sounding
S	Soil Sounding
SS	2"O.D. Split-Barrel Sample
2ST	2"O.D. Tin-Walled Tube Sample
3ST	3" O.D. Thin-Walled Tube Sample
VS	Vane Shear Test
DB	Diamond Bit
WS	Wash Sample
RB	Roller Bit
ST	Shelby Tube, 2" O.D. or 3" O.D.
CB	Carbide Bit
WOH	Weight of the Hammer

GRAIN SIZE TERMINOLOGY

RELATIVE DENSITY

CONSISTENCY

PLASTICITY

<u>Soil fraction</u>	<u>Particle size</u>	<u>Us standard sieve size</u>	<u>Term</u>	<u>"N" Value</u>	<u>Term</u>	<u>"N" Value</u>	<u>Term</u>	<u>Plastic Index</u>
Boulders	larger than 75 mm	Larger than 3"	Very Loose	0-5	Very Soft	0-3	None to Slight	0-4
Gravel	2mm to 75 mm	#10 to 75 mm	Loose	6-10	Soft	4-5	Slight	5-7
Coarse Sand	0.425 mm to 2 mm	#40 to #10	Medium Dense	11-30	Medium Stiff	6-10	Medium	8-22
Fine Sand	0.075mm to 0.425 mm	#200 to #40	Dense	31-50	Stiff	11-15	High/Very High	Over 22
Silt	0.002 mm to 0.075 mm	Smaller than #200	Very Dense	51+	Very Stiff	16-30		
Clay	Smaller than 0.002 mm	Smaller than #200			Hard	31+		

Note(s):

The penetration resistance, "N" Value, is the summation of the number of blows required to effect two successive 6-inch penetrations of the 2-inch split-barrel sampler. The sampler is driven with a 140-lb. weight falling 30-inches and is seated to a depth of 6-inches before commencing the standard penetration test.

Water level measurements shown on the boring logs represent conditions at the time indicated and may not reflect static levels, especially in cohesive soils

GME TESTING

3517 Focus Drive

Fort Wayne, IN 46818

(260) 497- 8127 • (877) 660-4GME • (260) 497- 0826 fax

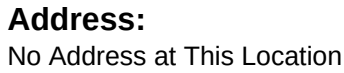
Division of **GEOTECHNICAL & MATERIALS ENGINEERS, INC.**

www.gmetesting.com

SOIL CLASSIFICATION CHART

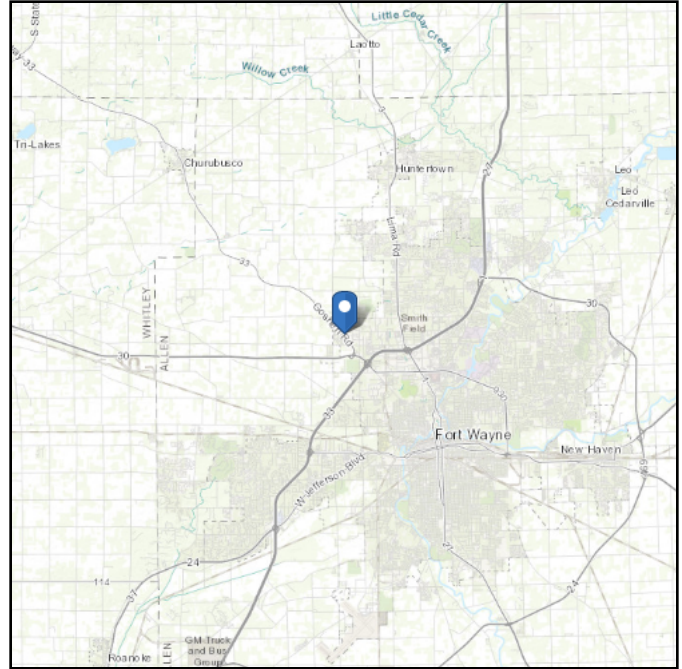
MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS
			GRAPH	LETTER	
COARSE GRAINED SOILS MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	GRAVEL AND GRAVELLY SOILS MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	CLEAN GRAVELS (LITTLE OR NO FINES)		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
				GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES (APPRECIABLE AMOUNT OF FINES)		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
				GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
	SAND AND SANDY SOILS MORE THAN 50% OF COARSE FRACTION PASSING ON NO. 4 SIEVE	CLEAN SANDS (LITTLE OR NO FINES)		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
				SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
		SANDS WITH FINES (APPRECIABLE AMOUNT OF FINES)		SM	SILTY SANDS, SAND - SILT MIXTURES
				SC	CLAYEY SANDS, SAND - CLAY MIXTURES
FINE GRAINED SOILS MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS LIQUID LIMIT LESS THAN 50			ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
				CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
	SILTS AND CLAYS LIQUID LIMIT GREATER THAN 50			MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
				CH	INORGANIC CLAYS OF HIGH PLASTICITY
				OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
HIGHLY ORGANIC SOILS				PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS



Standard: ASCE/SEI 7-10
Risk Category: III
Soil Class: D - Stiff Soil

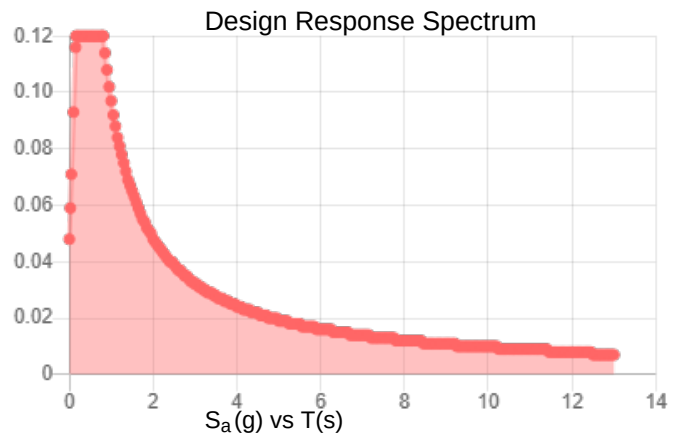
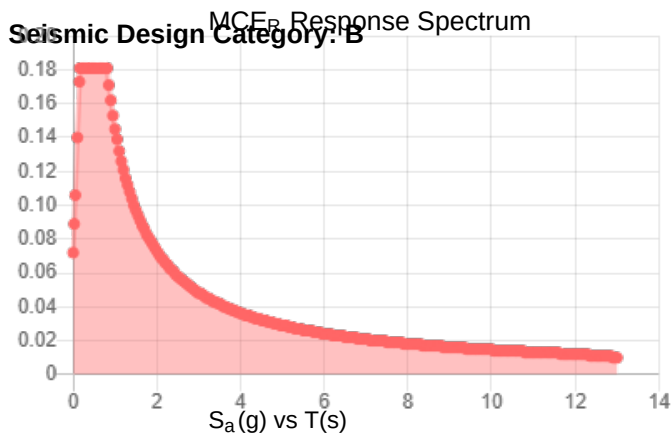
Longitude: -85.203559



Site Soil Class: D - Stiff Soil

Results:

S_S :	0.113	S_{D1} :	0.097
S_1 :	0.061	T_L :	12
F_a :	1.6	PGA :	0.053
F_v :	2.4	PGA _M :	0.085
S_{MS} :	0.181	F_{PGA} :	1.6
S_{M1} :	0.145	I_e :	1.25
S_{DS} :	0.12		



Data Accessed: Wed Oct 01 2025

Date Source:

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

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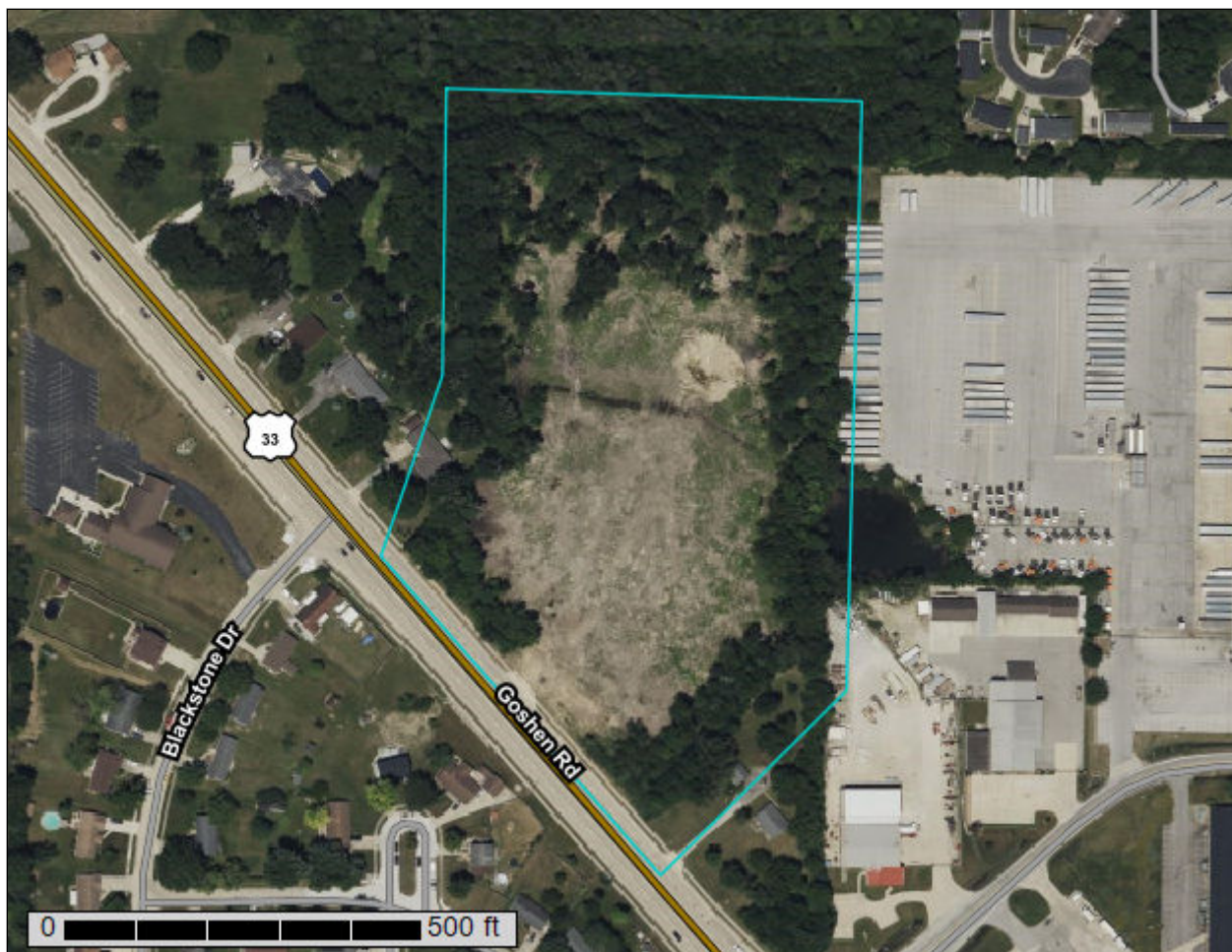
United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for **Allen County, Indiana**



October 1, 2025

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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Contents

Preface	2
How Soil Surveys Are Made	5
Soil Map	8
Soil Map.....	9
Legend.....	10
Map Unit Legend.....	11
Map Unit Descriptions.....	11
Allen County, Indiana.....	13
BmA—Blount loam, interlobate moraines, 0 to 2 percent slopes.....	13
MrB2—Glynwood silt loam, 2 to 6 percent slopes, eroded.....	14
Pe—Pewamo silty clay loam, 0 to 1 percent slopes.....	16
UepA—Urban land-Blount complex, 0 to 2 percent slopes.....	17
UetB2—Urban land-Glynwood complex, 2 to 6 percent slopes, eroded.....	19
UppA—Urban land-Pewamo complex, 0 to 1 percent slopes.....	21
References	24

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



Custom Soil Resource Report

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals

Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15,800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Allen County, Indiana
Survey Area Data: Version 27, Sep 17, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 18, 2022—Jun 21, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BmA	Blount loam, interlobate moraines, 0 to 2 percent slopes	7.8	61.0%
MrB2	Glynwood silt loam, 2 to 6 percent slopes, eroded	0.1	1.0%
Pe	Pewamo silty clay loam, 0 to 1 percent slopes	4.6	35.9%
UepA	Urban land-Blount complex, 0 to 2 percent slopes	0.1	0.6%
UetB2	Urban land-Glynwood complex, 2 to 6 percent slopes, eroded	0.0	0.2%
UppA	Urban land-Pewamo complex, 0 to 1 percent slopes	0.2	1.3%
Totals for Area of Interest		12.7	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not

mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Allen County, Indiana

BmA—Blount loam, interlobate moraines, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2yddh
Elevation: 700 to 1,050 feet
Mean annual precipitation: 34 to 42 inches
Mean annual air temperature: 46 to 52 degrees F
Frost-free period: 140 to 180 days
Farmland classification: Prime farmland if drained

Map Unit Composition

Blount and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Blount

Setting

Landform: Ground moraines, end moraines
Landform position (two-dimensional): Summit, footslope
Landform position (three-dimensional): Interfluve
Down-slope shape: Linear, concave
Across-slope shape: Linear
Parent material: Wisconsin till derived from limestone and shale

Typical profile

Ap - 0 to 9 inches: loam
Bt - 9 to 30 inches: clay
BC - 30 to 37 inches: clay loam
Cd - 37 to 79 inches: clay loam

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: 28 to 48 inches to densic material
Drainage class: Somewhat poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Low to moderately high
(0.01 to 0.20 in/hr)
Depth to water table: About 6 to 12 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 35 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 5.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2w
Hydrologic Soil Group: D
Ecological site: F111XB502IN - Wet Till Ridge
Hydric soil rating: No

Minor Components

Haskins

Percent of map unit: 9 percent
Landform: Ground moraines
Landform position (two-dimensional): Backslope, footslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F111XB502IN - Wet Till Ridge
Hydric soil rating: No

Glynwood

Percent of map unit: 6 percent
Landform: Ground moraines, end moraines
Landform position (two-dimensional): Shoulder, backslope, summit
Landform position (three-dimensional): Nose slope, side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Linear, convex
Ecological site: F111XE503IN - Till Ridge
Hydric soil rating: No

Pewamo

Percent of map unit: 5 percent
Landform: Depressions on ground moraines
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Linear
Across-slope shape: Concave
Ecological site: F111XB501IN - Till Depression
Hydric soil rating: Yes

MrB2—Glynwood silt loam, 2 to 6 percent slopes, eroded

Map Unit Setting

National map unit symbol: 2t6lq
Elevation: 700 to 1,320 feet
Mean annual precipitation: 34 to 42 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 140 to 180 days
Farmland classification: All areas are prime farmland

Map Unit Composition

Glynwood and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Glynwood

Setting

Landform: Ground moraines on till plains, end moraines on till plains

Landform position (two-dimensional): Shoulder, backslope, summit

Landform position (three-dimensional): Nose slope, side slope

Down-slope shape: Convex, linear

Across-slope shape: Linear, convex

Parent material: Wisconsin till derived from limestone and shale

Typical profile

Ap - 0 to 7 inches: silt loam

Bt - 7 to 25 inches: clay

BC - 25 to 29 inches: clay loam

Cd - 29 to 79 inches: clay loam

Properties and qualities

Slope: 2 to 6 percent

Depth to restrictive feature: 24 to 42 inches to densic material

Drainage class: Moderately well drained

Runoff class: High

Capacity of the most limiting layer to transmit water (Ksat): Low to moderately high
(0.01 to 0.20 in/hr)

Depth to water table: About 12 to 24 inches

Frequency of flooding: None

Frequency of ponding: None

Calcium carbonate, maximum content: 35 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Low (about 4.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: D

Ecological site: F111XE503IN - Till Ridge

Hydric soil rating: No

Minor Components

Rawson

Percent of map unit: 6 percent

Landform: End moraines on till plains, ground moraines on till plains

Landform position (two-dimensional): Shoulder, backslope, summit

Landform position (three-dimensional): Side slope, crest, nose slope

Down-slope shape: Convex, linear

Across-slope shape: Linear, convex

Ecological site: F111XE503IN - Till Ridge

Hydric soil rating: No

Blount

Percent of map unit: 5 percent

Landform: Ground moraines on till plains, end moraines on till plains

Landform position (two-dimensional): Summit, backslope, footslope

Landform position (three-dimensional): Interfluve, side slope

Down-slope shape: Linear

Across-slope shape: Linear

Custom Soil Resource Report

Ecological site: F111XB502IN - Wet Till Ridge

Hydric soil rating: No

Pewamo

Percent of map unit: 4 percent

Landform: Ground moraines on till plains, end moraines on till plains

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Base slope

Down-slope shape: Linear

Across-slope shape: Concave

Ecological site: F111XB501IN - Till Depression

Hydric soil rating: Yes

Pe—Pewamo silty clay loam, 0 to 1 percent slopes

Map Unit Setting

National map unit symbol: 2t6lv

Elevation: 700 to 1,300 feet

Mean annual precipitation: 32 to 42 inches

Mean annual air temperature: 48 to 54 degrees F

Frost-free period: 140 to 180 days

Farmland classification: Prime farmland if drained

Map Unit Composition

Pewamo and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Pewamo

Setting

Landform: Depressions on till plains, drainageways on till plains

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Base slope

Down-slope shape: Concave, linear

Across-slope shape: Concave

Parent material: Wisconsin till derived from limestone and shale

Typical profile

Ap - 0 to 11 inches: silty clay loam

Btg1 - 11 to 34 inches: silty clay

Btg2 - 34 to 47 inches: silty clay

BCg - 47 to 57 inches: clay loam

Cg - 57 to 79 inches: clay loam

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Very poorly drained

Runoff class: Negligible

Custom Soil Resource Report

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: About 0 to 12 inches

Frequency of flooding: None

Frequency of ponding: Frequent

Calcium carbonate, maximum content: 30 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Moderate (about 8.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2w

Hydrologic Soil Group: C/D

Ecological site: F111XB501IN - Till Depression

Hydric soil rating: Yes

Minor Components

Blount

Percent of map unit: 9 percent

Landform: Ground moraines on till plains, end moraines on till plains

Landform position (two-dimensional): Footslope

Landform position (three-dimensional): Interfluve

Down-slope shape: Linear

Across-slope shape: Linear

Ecological site: F111XB502IN - Wet Till Ridge

Hydric soil rating: No

Minster

Percent of map unit: 6 percent

Landform: Depressions on till plains

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Base slope

Down-slope shape: Concave

Across-slope shape: Concave

Ecological site: F111XB101IN - Lacustrine Flatwood

Hydric soil rating: Yes

UepA—Urban land-Blount complex, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2yc4g

Elevation: 700 to 1,300 feet

Mean annual precipitation: 34 to 42 inches

Mean annual air temperature: 48 to 54 degrees F

Frost-free period: 140 to 180 days

Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 65 percent

Custom Soil Resource Report

Blount and similar soils: 25 percent

Minor components: 10 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Urban Land

Setting

Landform: Ground moraines, end moraines, moraines

Landform position (two-dimensional): Summit, shoulder

Landform position (three-dimensional): Interfluve

Down-slope shape: Linear

Across-slope shape: Linear

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 8

Hydric soil rating: No

Description of Blount

Setting

Landform: End moraines, ground moraines, moraines

Landform position (two-dimensional): Summit, shoulder

Landform position (three-dimensional): Interfluve

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Wisconsin till derived from limestone and shale

Typical profile

Ap - 0 to 10 inches: silt loam

Bt - 10 to 33 inches: silty clay

BC - 33 to 39 inches: clay loam

Cd - 39 to 79 inches: clay loam

Properties and qualities

Slope: 0 to 2 percent

Depth to restrictive feature: 31 to 54 inches to densic material

Drainage class: Somewhat poorly drained

Runoff class: High

*Capacity of the most limiting layer to transmit water (Ksat): Low to moderately high
(0.01 to 0.20 in/hr)*

Depth to water table: About 6 to 12 inches

Frequency of flooding: None

Frequency of ponding: None

Calcium carbonate, maximum content: 35 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Moderate (about 6.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2w

Hydrologic Soil Group: D

Ecological site: F111XB502IN - Wet Till Ridge

Hydric soil rating: No

Minor Components

Pewamo

Percent of map unit: 6 percent
Landform: Depressions
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Concave
Across-slope shape: Concave, linear
Ecological site: F111XB501IN - Till Depression
Hydric soil rating: Yes

Glynwood

Percent of map unit: 4 percent
Landform: End moraines, ground moraines, moraines
Landform position (two-dimensional): Shoulder, backslope
Landform position (three-dimensional): Nose slope, side slope
Down-slope shape: Convex, linear
Across-slope shape: Linear
Ecological site: F111XE503IN - Till Ridge
Hydric soil rating: No

UetB2—Urban land-Glynwood complex, 2 to 6 percent slopes, eroded

Map Unit Setting

National map unit symbol: 2z69g
Elevation: 700 to 1,320 feet
Mean annual precipitation: 34 to 42 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 140 to 180 days
Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 60 percent
Glynwood and similar soils: 30 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Urban Land

Setting

Landform: Moraines, end moraines, ground moraines
Landform position (two-dimensional): Shoulder, backslope, summit
Landform position (three-dimensional): Nose slope, side slope
Down-slope shape: Convex, linear
Across-slope shape: Linear, convex

Interpretive groups

Land capability classification (irrigated): None specified

Custom Soil Resource Report

Land capability classification (nonirrigated): 8

Hydric soil rating: No

Description of Glynwood

Setting

Landform: Ground moraines on till plains, end moraines on till plains

Landform position (two-dimensional): Shoulder, backslope, summit

Landform position (three-dimensional): Nose slope, side slope

Down-slope shape: Convex, linear

Across-slope shape: Linear, convex

Parent material: Wisconsin till derived from limestone and shale

Typical profile

Ap - 0 to 7 inches: silt loam

Bt - 7 to 25 inches: clay

BC - 25 to 29 inches: clay loam

Cd - 29 to 79 inches: clay loam

Properties and qualities

Slope: 2 to 6 percent

Depth to restrictive feature: 24 to 42 inches to densic material

Drainage class: Moderately well drained

Runoff class: High

Capacity of the most limiting layer to transmit water (Ksat): Low to moderately high
(0.01 to 0.20 in/hr)

Depth to water table: About 12 to 24 inches

Frequency of flooding: None

Frequency of ponding: None

Calcium carbonate, maximum content: 35 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water supply, 0 to 60 inches: Low (about 4.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: D

Ecological site: F111XE503IN - Till Ridge

Hydric soil rating: No

Minor Components

Rawson

Percent of map unit: 4 percent

Landform: End moraines on till plains, ground moraines on till plains

Landform position (two-dimensional): Shoulder, backslope, summit

Landform position (three-dimensional): Side slope, crest, nose slope

Down-slope shape: Convex, linear

Across-slope shape: Linear, convex

Ecological site: F111XE503IN - Till Ridge

Hydric soil rating: No

Pewamo

Percent of map unit: 3 percent

Landform: Ground moraines on till plains, end moraines on till plains

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Base slope

Custom Soil Resource Report

Down-slope shape: Linear
Across-slope shape: Concave
Ecological site: F111XB501IN - Till Depression
Hydric soil rating: Yes

Blount

Percent of map unit: 3 percent
Landform: Ground moraines on till plains, end moraines on till plains
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Interfluve, side slope
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F111XB502IN - Wet Till Ridge
Hydric soil rating: No

UppA—Urban land-Pewamo complex, 0 to 1 percent slopes

Map Unit Setting

National map unit symbol: 2yc4t
Elevation: 700 to 1,300 feet
Mean annual precipitation: 32 to 42 inches
Mean annual air temperature: 48 to 54 degrees F
Frost-free period: 140 to 180 days
Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 60 percent
Pewamo and similar soils: 30 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Urban Land

Setting

Landform: Drainageways, depressions, moraines
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Linear, concave
Across-slope shape: Concave

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8
Hydric soil rating: No

Description of Pewamo

Setting

Landform: Depressions on till plains, drainageways on till plains
Landform position (two-dimensional): Toeslope

Custom Soil Resource Report

Landform position (three-dimensional): Base slope
Down-slope shape: Concave, linear
Across-slope shape: Concave
Parent material: Wisconsin till derived from limestone and shale

Typical profile

Ap - 0 to 11 inches: silty clay loam
Btg1 - 11 to 34 inches: silty clay
Btg2 - 34 to 47 inches: silty clay
BCg - 47 to 57 inches: clay loam
Cg - 57 to 79 inches: clay loam

Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: About 0 to 12 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Calcium carbonate, maximum content: 30 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 8.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2w
Hydrologic Soil Group: C/D
Ecological site: F111XB501IN - Till Depression
Hydric soil rating: Yes

Minor Components

Blount

Percent of map unit: 6 percent
Landform: Ground moraines on till plains, end moraines on till plains
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Interfluve
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F111XB502IN - Wet Till Ridge
Hydric soil rating: No

Minster

Percent of map unit: 4 percent
Landform: Depressions on till plains
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Concave
Across-slope shape: Concave
Ecological site: F111XB101IN - Lacustrine Flatwood
Hydric soil rating: Yes

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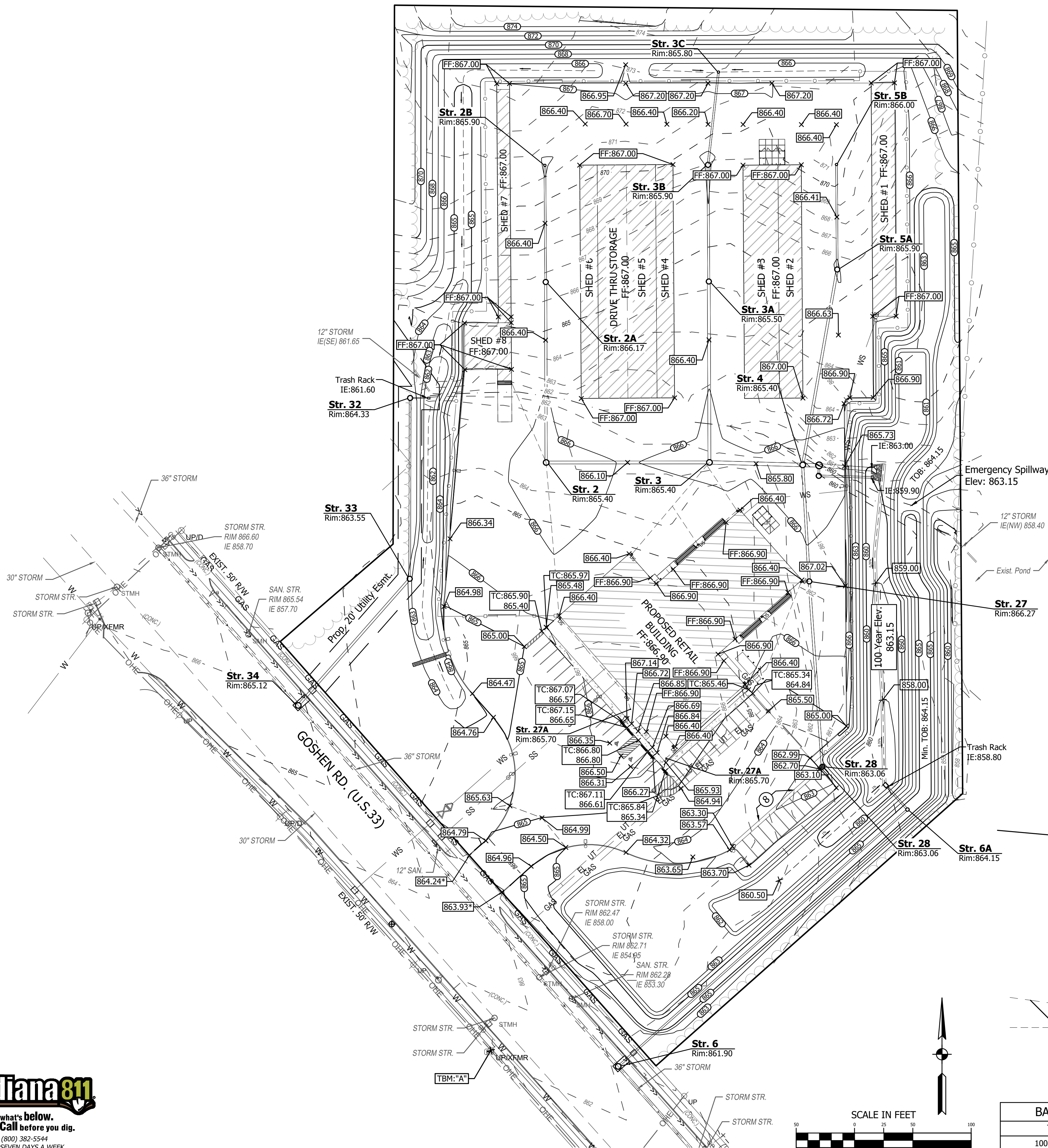
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C:\PROJECTS\2025\25-1610 VINTAGE ARCHONICS BIG C LUMBER\20 CIVIL\24 CAD\PLANS\25-1610 DRAIN AND GRADING.DWG DGP-01 JOHN WYLE 6.3.2026 4:07 PM



GRADING NOTES

- Contractor shall follow the latest Fort Wayne and the State of Indiana "CSGP" Soil Erosion Practices, See Erosion Control Sheets.
- All topsoil shall be removed and temporarily stockpiled on-site at a location approved by the Owner. A minimum of four (4) inches of topsoil shall be placed on all disturbed areas outside the building and parking areas.
- All exposed subgrade shall be proof-rolled and witnessed by a Geotechnical Engineer or qualified representative to determine unsuitable soil locations prior to any paving operations. Any unsuitable soil shall be excavated, backfilled, and compacted with suitable material in accordance with the plans and capable of supporting the anticipated loadings of the project.
- Positive drainage shall be maintained to prevent any ponding of water or encroachment onto adjacent properties.
- Proposed contours show grading intent only. Contractor shall use proposed spot grade elevations and check all grade stakes to ensure positive drainage to stormwater system is achieved. Contact Engineer if additional grades are needed or if any discrepancies or conflicts which become apparent before or during construction are found so that clarification or redesign may occur.
- Excess and unsuitable soil shall be removed from project site to another site. It is the Contractor's responsibility to implement erosion control measures and any required permits for the relocated material.

LEGEND

- Proposed Contours
- Existing Contours
- Proposed Spot Elevation
- Match Existing Elevation
- FF: Finish Floor Elevation
- TC: Top of Curb
- IE: Pipe Invert Elevation
- Concrete Gutter, per Construction Details
- Storm Inlet or Manhole w/ Storm Pipe
- Storm Pipe for Downspout

CONC. CHANNEL
(NOT TO SCALE)

EMERGENCY SPILLWAY
(NOT TO SCALE)

BASIN SECTION, TYPICAL
(NOT TO SCALE)

BASIN ELEVATION TABLE			
Top of Bank Elev.	A	864.15	
100 Year Water Elev.	B	863.15	

BIG C LUMBER, INC.

BIG C LUMBER

4748 US 33 FORT WAYNE, IN 46818

GRADING PLAN

FOR REVIEW

GRADING
ADJUSTMENT
PRELIMINARY
06.05.2026

ISSUE DATE: 05.07.2026
PM: TE QA / QC: WLE

ISSUANCE / REVISION DATE
ORIGINAL SHEET IS 24"x36". USE DIMENSIONS
SHOWN, DO NOT SCALE DRAWINGS.

PROJECT NO: 25-1610

C3.0



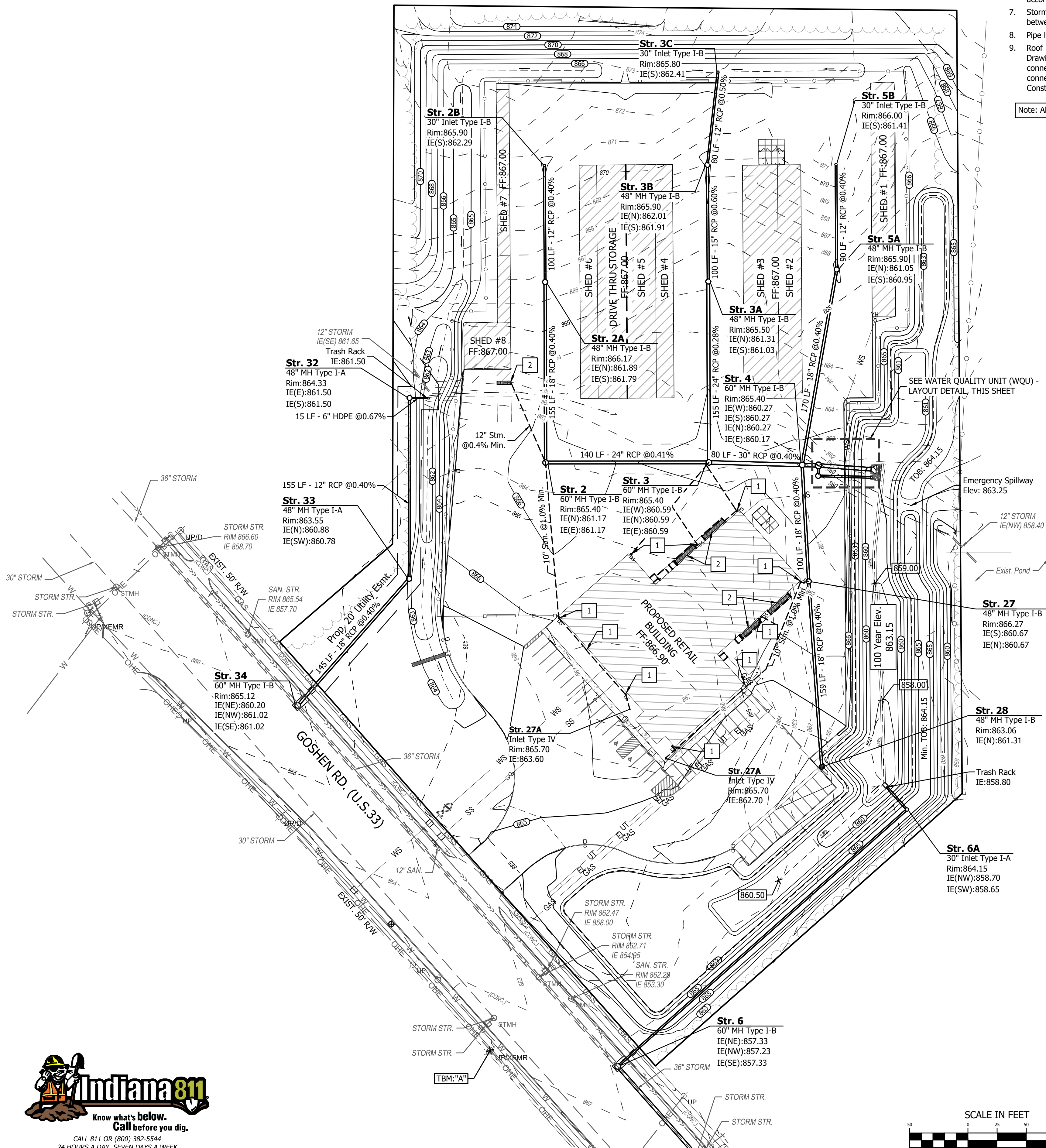
229 W. Berry Street, Suite 100
Fort Wayne, IN 46802
T 260.218.2500
abonmarche.com

Surveying
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C:\PROJECTS\2025\25-1610 VINTAGE ARCHONICS BIG C LUMBER\20 CIVIL\24 CAD\PLANS\25-1610 STORM.DWG ST-01 JOHN WYLLIE 6.5.2026 4:07 PM



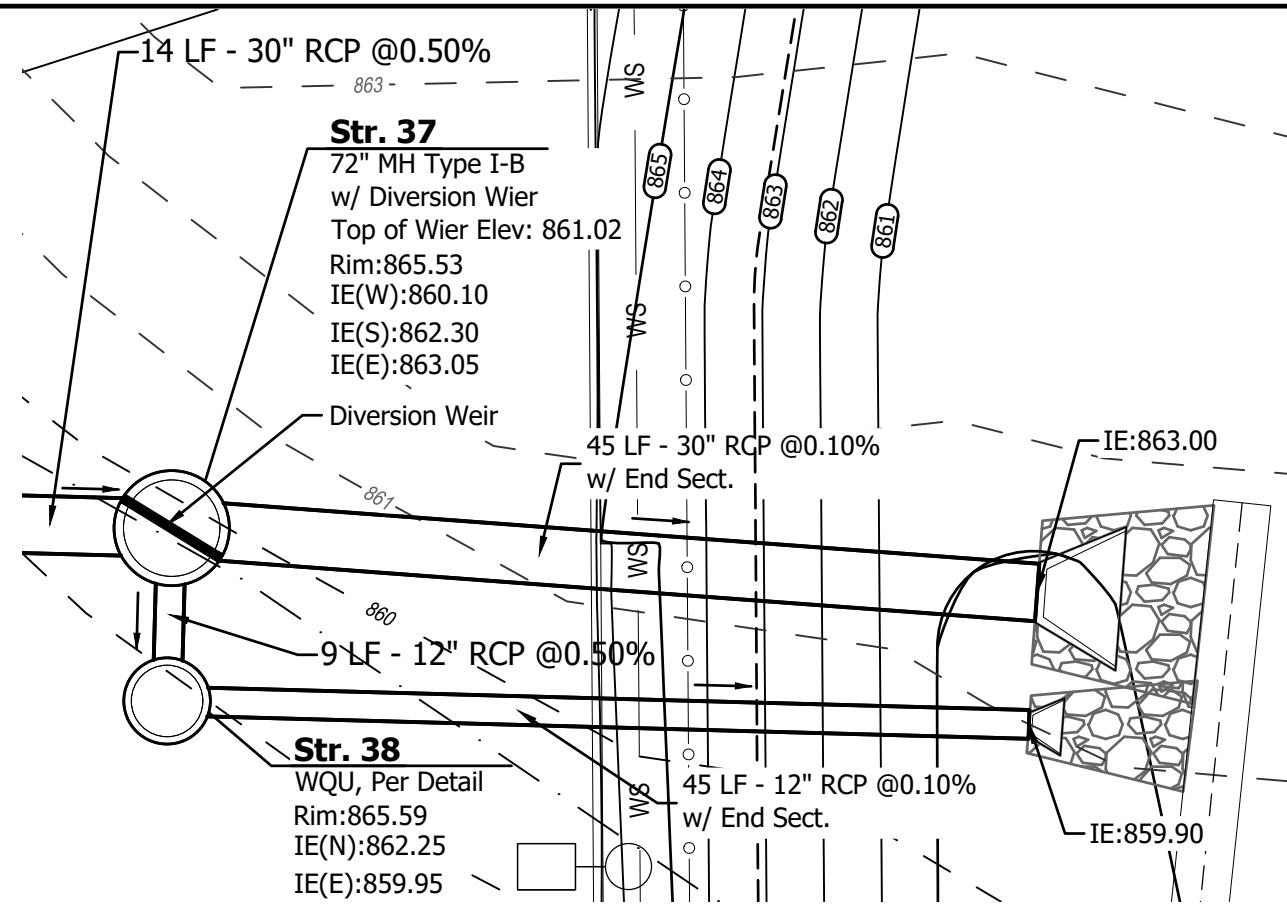
STORM SEWER NOTES

- All proposed Storm pipe shall be in accordance with the County Surveyor's Office and INDOT Standards and Specifications.
- All storm pipe within the public right-of-way and below pavement shall be CL-IV (Reinforced Concrete Pipe) RCP.
- RCP Pipe shall conform to ASTM C-76 standards.
- Class "B" Bedding shall be used for RCP pipe.
- All Storm pipe 10" and smaller shall be SDR 35 PVC OR ADS N-12 HDPE unless noted otherwise.
- Pipe shall be bedded per pipe manufacturer, and backfilled with suitable clean granular material in accordance with the plans, see pipe trench detail, see sheet C5.2.
- Storm pipe shall maintain 10'-0" minimum horizontal and 18" minimum vertical separation between potable water pipes.
- Pipe lengths are measured from center of structures.
- Roof downspouts storm piping: Prior to construction, Contractor shall review Architectural Drawings accounting for all downspouts. Downspout storm piping shall be intercepted and connected to the storm sewer. Provide all necessary piping, fittings, and transition adapters to connect downspout piping to the storm sewer. See Typical Downspout Connection per Construction Details.

Note: All Storm End Sections shall have an Energy Dissipator, See Erosion Control Details.

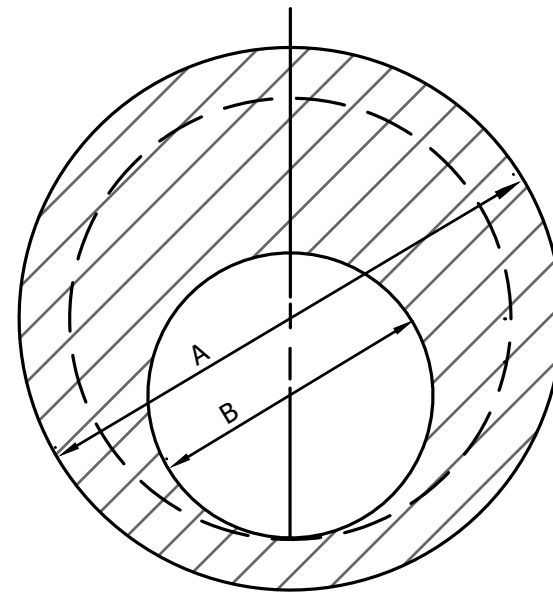
LEGEND

- Proposed Contours
- Existing Contours
- Swale Centerline
- Storm Inlet or Manhole w/ Storm Pipe
- Storm Pipe for Downspout
- Downspout Connection to Building/Storm Sewer w/ Appropriate Fittings, Coordinate w/ Building Plans
- Trench Drain per Construction Details. Connection to Downspout Drains w/ Appropriate Fittings, See Arch' Drawings for Detail.



WATER QUALITY UNIT (WQU) - LAYOUT DETAIL

SCALE: 1" = 10'

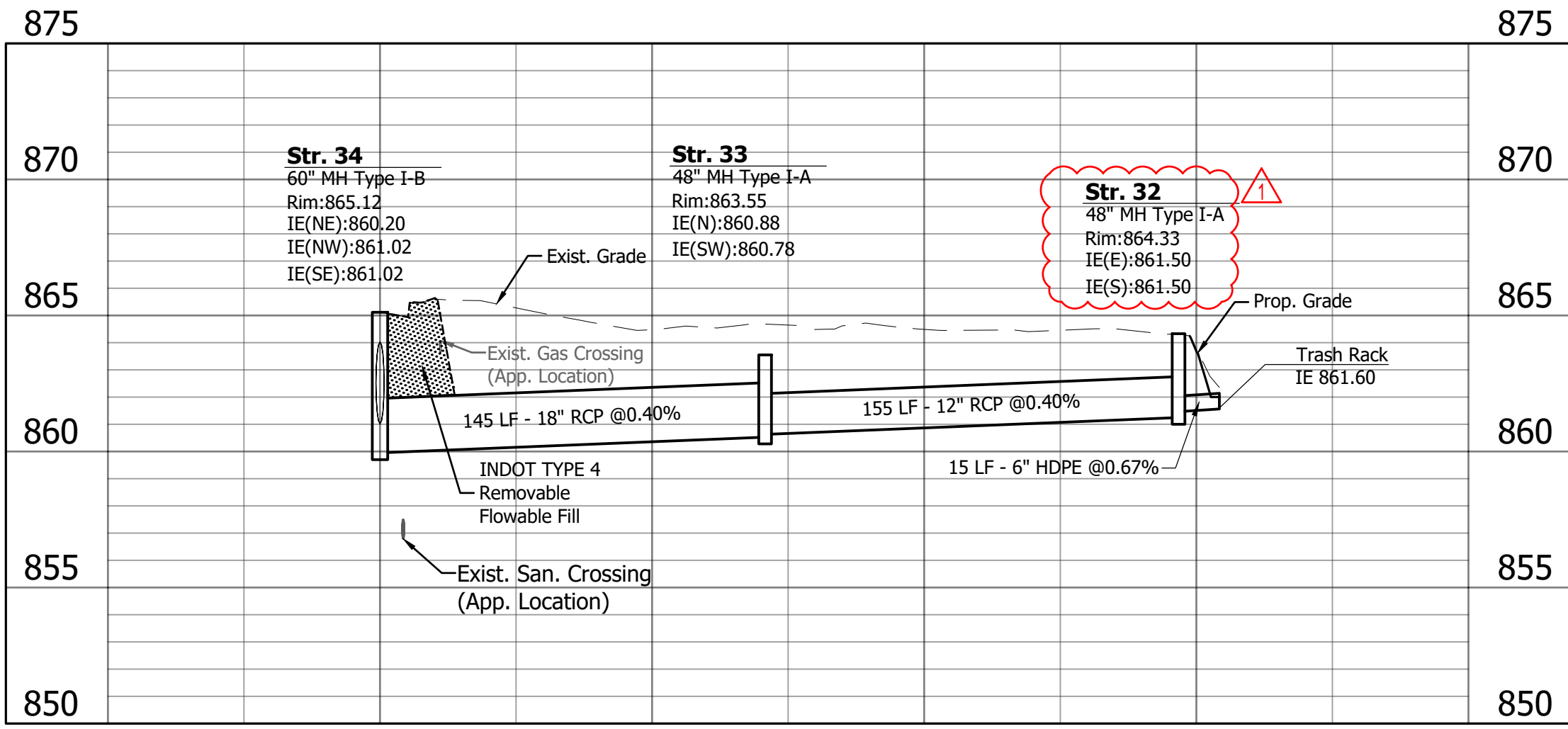


STR#	A (Pipe Ø)	B (Opening Ø)
6A	12"	4.7"

Note: Stainless steel / galvanized orifice plate, thickness = 6 mm. Plate to be mounted to interior face of concrete structure with stainless steel anchors (Typ.), sealed with rubberized sealant or non-shrink grout. Orifice opening to be centered on outlet pipe.

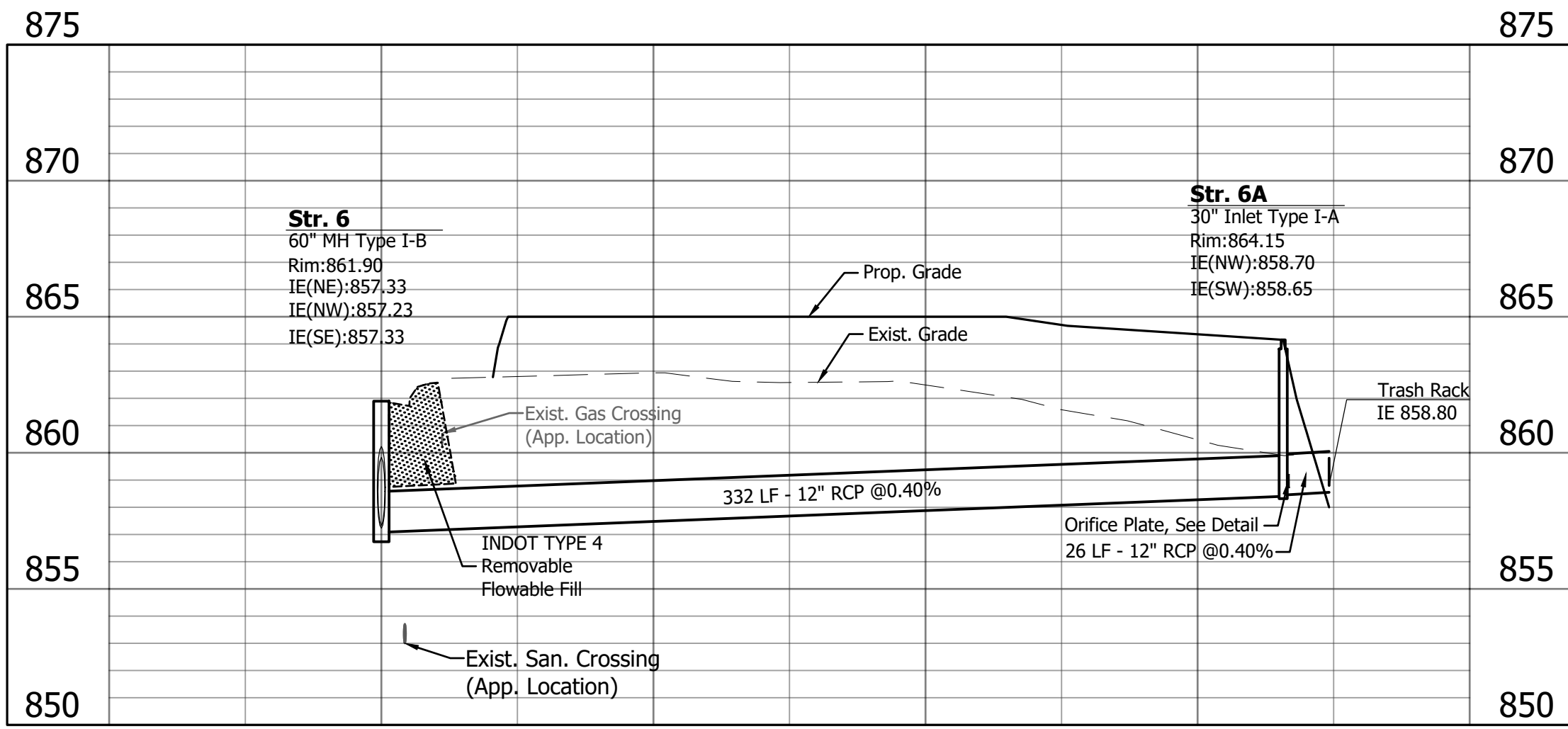
ORIFICE PLATE

(NOT TO SCALE)



OFFSITE STORM ROUTE

Scale: Vert. 1" = 5'
Horz. 1" = 50'



BASIN OUTLET PROFILE

Scale: Vert. 1" = 5'
Horz. 1" = 50'

BIG C LUMBER, INC.

BIG C LUMBER

4748 US 33 FORT WAYNE, IN 46818

STORM PLAN

FOR REVIEW

GRADING
ADJUSTMENT
PRELIMINARY
06.05.2026

ISSUE DATE: 05.19.2026
PM: TE QA / QC: WLE

1 STORM LABEL ADDED 05.19.2026
ISSUANCE / REVISION DATE
ORIGINAL SHEET IS 24"X36", USE DIMENSIONS SHOWN, DO NOT SCALE DRAWINGS.

PROJECT NO: 25-1610

C3.1

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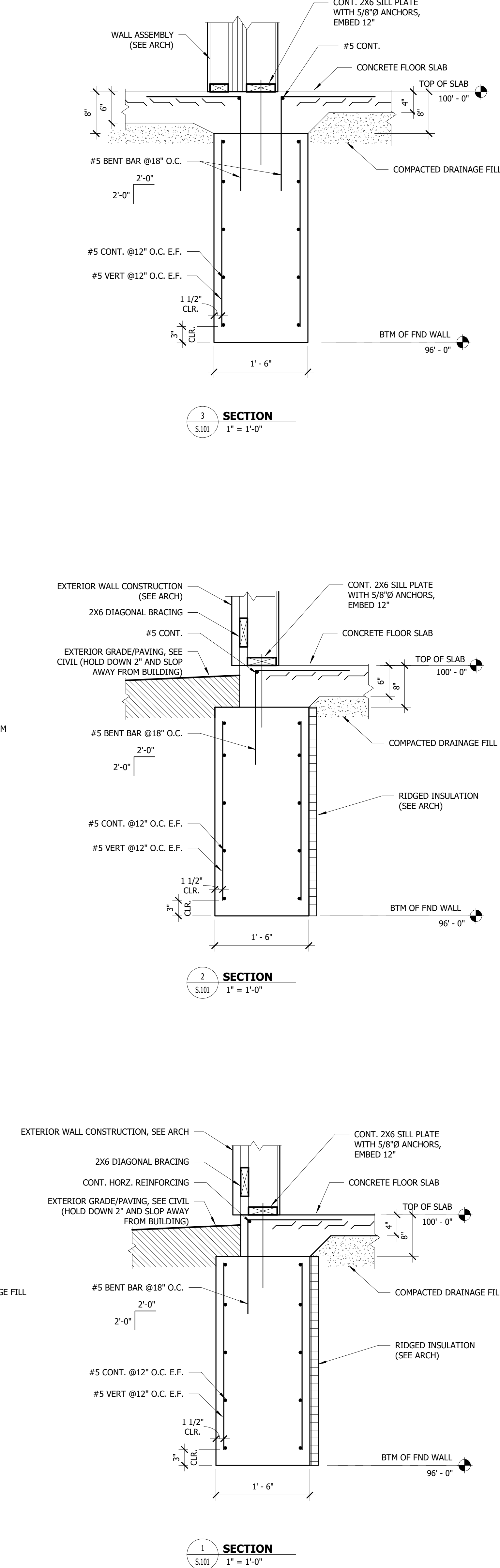
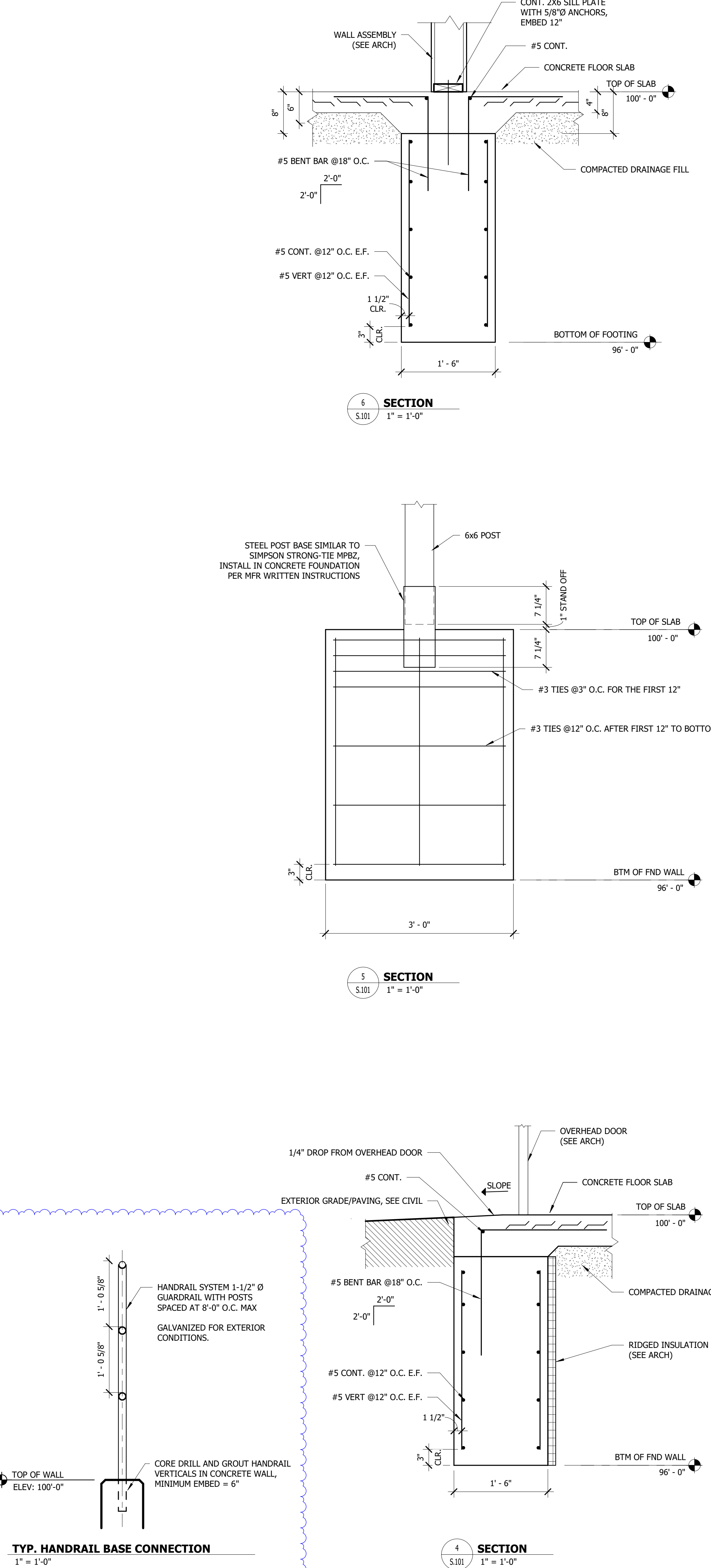
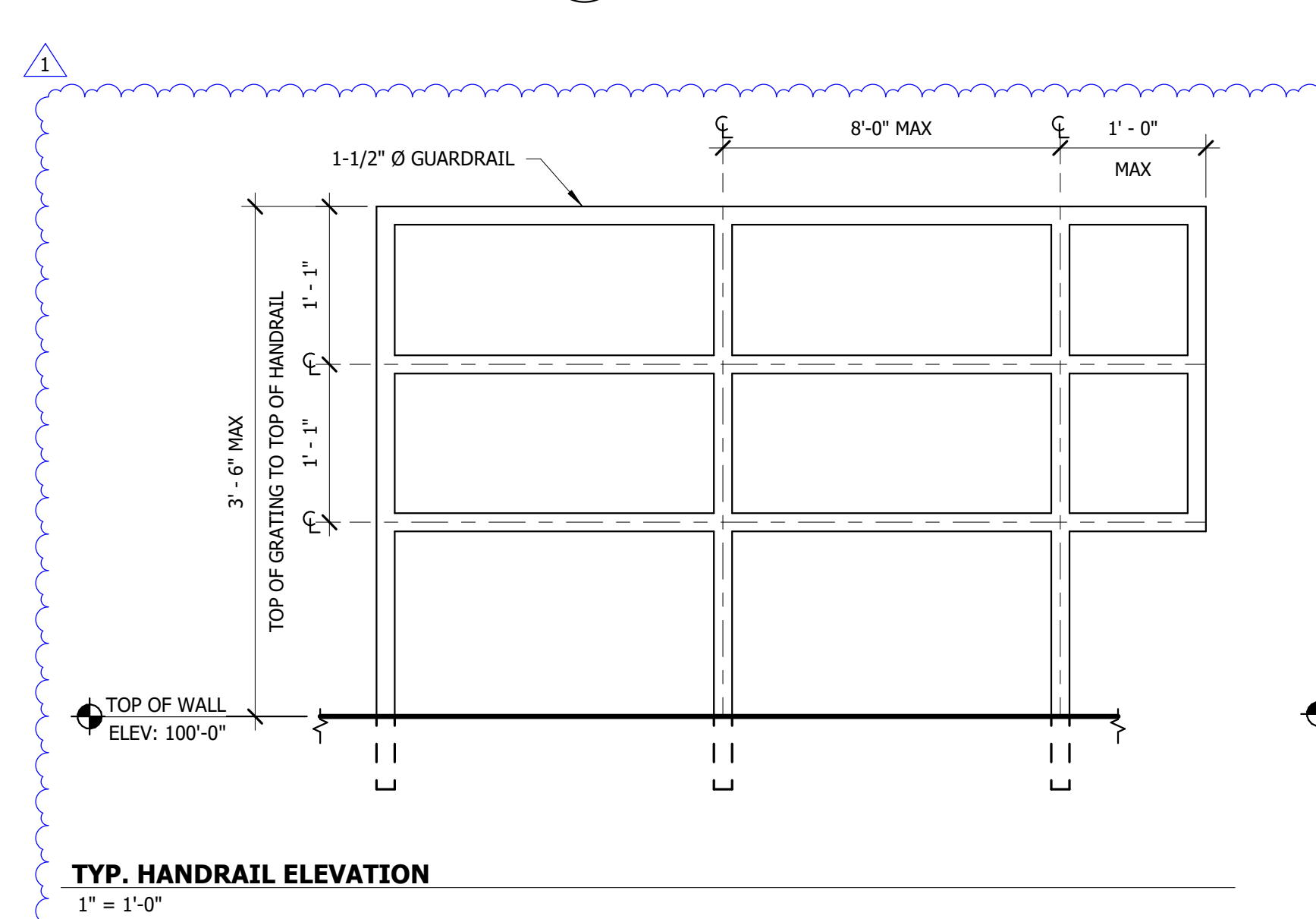
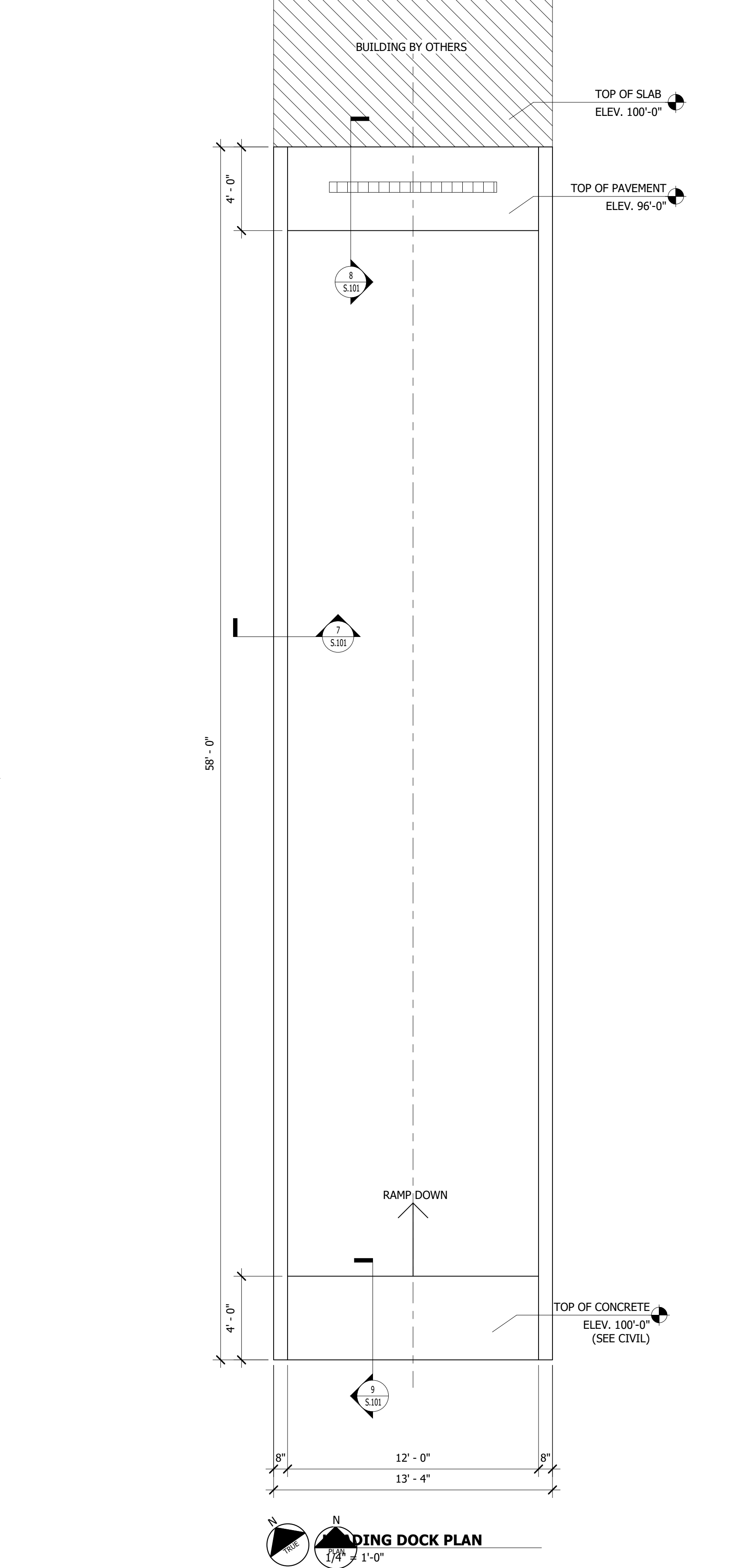
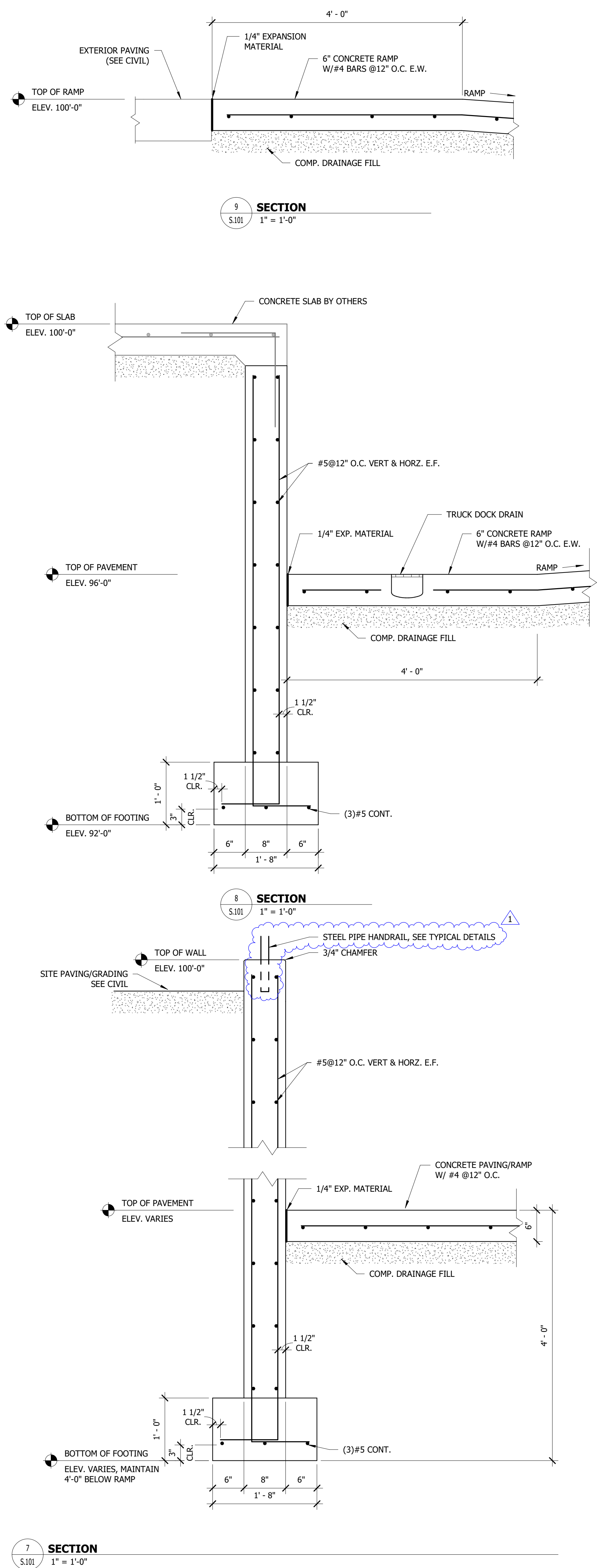
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DISCIPLINE
G - GENERAL
C - CIVIL
L - LANDSCAPE
S - STRUCTURAL
A - ARCHITECTURAL
I - INTERIORS
P - PLUMBING
F - FIRE PROTECTION
M - MECHANICAL
E - ELECTRICAL

PHASE
SHEET TYPE
FLOOR LEVEL
SHEET COUNT

A2.2.2.2

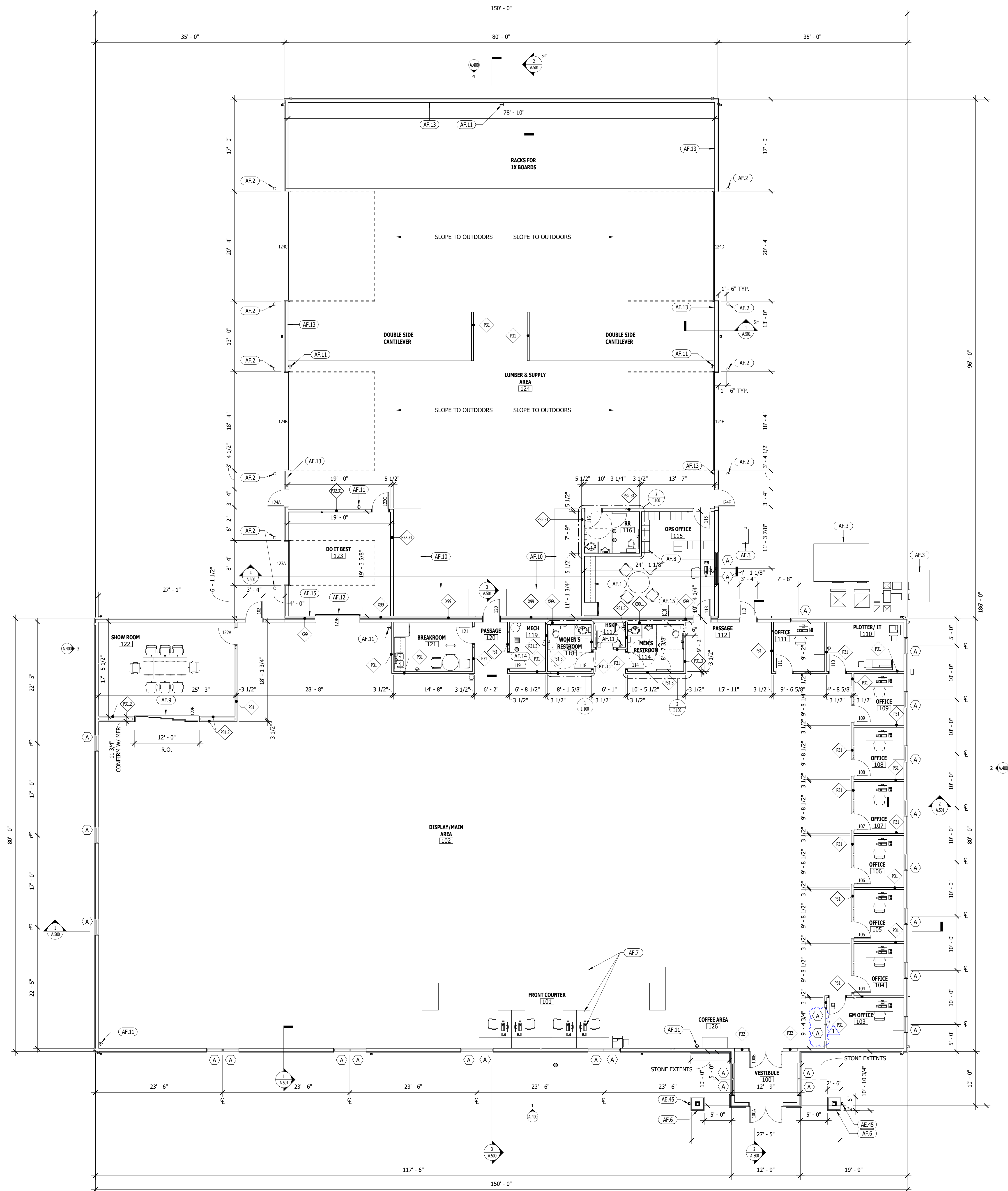


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PHASE
SHEET TYPE
FLOOR LEVEL
SHEET COUNT

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FLOOR PLAN FIRST FLOOR
1/8" = 1'-0"

GENERAL CONSTRUCTION NOTES

- A. **DO NOT SCALE THE DRAWINGS**
PLEASE FORWARD ALL QUESTIONS REGARDING CLARIFICATION OF DIMENSIONS TO THE ARCHITECT/ENGINEER FOR RESOLUTION.
- B. **PRIOR TO EXECUTION OF BIDS, CONTRACTORS SHALL:**
 1. THOROUGHLY REVIEW THE COMPLETE SET OF CONSTRUCTION DOCUMENTS TO GAIN AN UNDERSTANDING OF THE SCOPE OF WORK DESCRIBED THEREIN.
 2. CONDUCT A FIELD VISIT OF THE SITE CONDITIONS RELATIVE TO THE SCOPE OF WORK AS DESCRIBED IN THE SET OF CONSTRUCTION DOCUMENTS.
 3. IMMEDIATELY NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BETWEEN THE INFORMATION INCLUDED IN THE SET OF THE CONSTRUCTION DOCUMENTS AND THE FIELD VERIFIED SITE CONDITIONS FOR RESOLUTION.
- C. **PRIOR TO CREATION OF SHOP DRAWINGS, CONTRACTORS SHALL:**
 1. FIELD VERIFY DIMENSIONS RELATIVE TO THE WORK.
 2. FIELD VERIFY SITE CONDITIONS RELATIVE TO THE WORK.
- D. **PRIOR TO MATERIAL FABRICATION, CONTRACTORS SHALL:**
 1. CREATE AND SUBMIT MATERIAL AND EQUIPMENT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER, REFLECTING FIELD VERIFIED CONDITIONS AND DIMENSIONS, AND PROVIDING ALL INFORMATION, INCLUDING A SIGNED AUTHORIZATION INDICATING THAT FIELD DIMENSIONS WERE OBTAINED AND ARE ACCURATE TO THE BEST OF THEIR KNOWLEDGE, AS REQUIRED FOR APPROVAL.
 2. RECEIVE RETURNED SHOP DRAWINGS MARKED APPROVED TO PROCEED WITH FABRICATION AND INSTALLATION BY ARCHITECT/ENGINEER.
- E. **PRIOR TO CONSTRUCTION OR MATERIAL INSTALLATION, THE CONTRACTOR SHALL:**
 1. OBTAIN ALL PERMITS AS REQUIRED TO PERFORM THE SCOPE OF WORK OUTLINED IN THE CONSTRUCTION DOCUMENTS.
 2. KEEP (AND VISIBLY POST) ALL PERMITS ON THE JOBSITE AS DIRECTED BY THE AUTHORITY HAVING JURISDICTION.
 3. LAY OUT AND MARK ALL WALLS AND OPENINGS.
 4. IMMEDIATELY NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BETWEEN THE INFORMATION IN THE SET OF THE CONSTRUCTION DOCUMENTS AND THE FIELD VERIFIED SITE CONDITIONS FOR RESOLUTION.
- F. **CODE COMPLIANCE, THE CONTRACTORS SHALL**
PERFORM THE SCOPE OF WORK OUTLINED IN THE SET OF CONSTRUCTION DOCUMENTS IN COMPLIANCE WITH THE CODES LISTED IN "LIFE-SAFETY BUILDING DATA".
- G. **FIRESTOPPING - COMMON SINGLE CONTRACTOR**
UTILIZE THE SAME CONTRACTOR FOR THE FIRESTOPPING SCOPE OF WORK. SEE THE FIRESTOPPING NOTES ON THE LIFE SAFETY PLAN FOR MORE INFORMATION.
- H. **REGARDING BLOCKING FOR EQUIPMENT, CONTRACTOR SHALL:**
COORDINATE ALL WORK WITH THE EQUIPMENT MANUFACTURER TO PROVIDE AND INSTALL WALL BLOCKING FOR SUPPORT OF THE EQUIPMENT ENSURING ROUGH-IN CLEARANCE REQUIREMENTS ARE MET FOR EQUIPMENT INSTALLATION AND USE.
- I. **REGARDING NOTES AND DETAILS, BOTH:**
 - a. MAY BE GENERALIZED.
 - b. SHALL SERVE TO AID THE CONTRACTOR IN EVALUATION OF THIS WORK.
 - c. SHALL NOT BE HELD TO BE ALL INCLUSIVE.
- J. **REGARDING ALTERATIONS, PATCHING AND PREPARATION, CONTRACTORS SHALL:**
PERFORM FIELD ALTERATIONS, PATCHING AND PREPARATION FOR ALL NEW WORK AS REQUIRED WHETHER OR NOT IT IS SPECIFICALLY NOTED IN THESE DRAWINGS.
- K. **REGARDING INSTALLATION REQUIREMENTS, CONTRACTORS SHALL:**
CONSULT WITH PRODUCT MANUFACTURERS FOR INSTALLATION REQUIREMENTS.

GENERAL DIMENSION STRING NOTES

DIMENSION STRINGS REPRESENT THE FOLLOWING (UNLESS NOTED OTHERWISE):

EXTERIOR STUD WALL FRAMING

OUTSIDE FACE OF STUD AND OUTSIDE FACE OF CONCRETE FLOOR ARE IN VERTICAL ALIGNMENT.

INTERIOR STUD WALL FRAMING

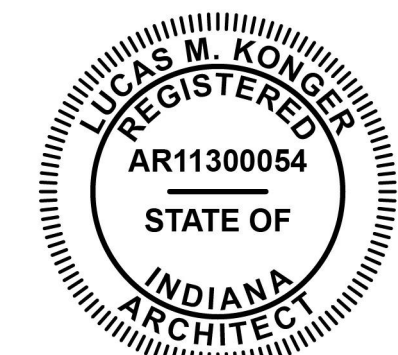
1. ACTUAL MATERIAL SIZE WIDTHS IN LIEU OF NOMINAL.
2. FACE OF STUD TO FACE OF STUD.
3. REGARDING INTERIOR DOOR FRAMES:
 - A. THE HINGE SIDE ADJACENT TO WALL INTERSECTIONS ARE TO BE HELD A MINIMUM OF 4-5/8" OFF STUDS TO DOOR FRAME.
 - B. THE LATCH SIDE ADJACENT TO THE NEAREST FINISHED CORNER ARE TO BE HELD A MINIMUM OF 10-5/8" OFF THE STUDS FOR COMPLIANCE WITH ACCESSIBILITY REQUIREMENTS.

ARCHITECTURAL PLAN KEYNOTES

- AF.1 COUNTERS/CASEWORK AND APPLIANCES BY OWNER.
- AF.2 PAINTED METAL BOLLARDS ON EACH SIDE OF OVERHEAD DOORS ONLY ON THE EXTERIOR SIDE.
- AF.3 HVAC/MECHANICAL GROUND UNITS. SEE MECHANICAL DRAWINGS FOR FURTHER INFORMATION. COORDINATE WITH CIVIL.
- AF.6 COLUMN WITH ADHERED STONE VENEER WITH STONE SILL AT COLUMN BASE. SEE ELEVATIONS, SECTIONS & STRUCTURAL DRAWINGS FOR FURTHER INFORMATION.
- AF.7 FRONT COUNTER CASEWORK, FURNITURE, COMPUTER EQUIPMENT BY OWNER.
- AF.8 LOCKERS BY OWNER.
- AF.9 TELESCOPING DOOR AT CONFERENCE ROOM DOOR. BASIS OF DESIGN: BY ANDERSON LIFT SLIDE STYLE DOOR, 5 PANEL, STRAIGHT CONFIGURATION, IN A +/-12'-0" OPENING. SEE DOOR SCHEDULE FOR SIZE.
- AF.10 TRIM RACKS BY OWNER.
- AF.11 FIRE EXTINGUISHER BY OWNER.
- AF.12 FIRE RATED OVERHEAD ROLLING DOOR WITHOUT GLAZED OPENING. BASIS OF DESIGN: OVERHEAD DOOR COMPANY, FIREKING MODEL#631 OR MODEL#635 IF INSULATION FOR SOUND IS REQUIRED, OR EQUAL. COORDINATE WITH OWNER FOR FINAL SELECTION.
- AF.13 NO FINISH MATERIAL ON EXTERIOR WALL OF INTERIOR FACE IN SHOP AREA U.N.O. PLUMBING EQUIPMENT, REFER TO PLUMBING DRAWINGS FOR FURTHER INFORMATION.
- AF.15 PROVIDE 10 MIL VAPOR BARRIER ONLY AT CONDITIONED ROOM SIDE. EXTEND TO VAPOR BARRIER TO UNDERSIDE OF TRUSS - TAPE & SEAL. REFER TO NOTE AT WALL SECTION.

VINTAGE ARCHONICS
ARCHITECTURE
ENGINEERING
INTERIOR DESIGN

8205 Lima Road
Fort Wayne, Indiana 46818
260.489.3543
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Lucas M. Kowner

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4700 Goshen Rd, Fort Wayne, Indiana 46818

Current Revision: 1
REVISIONS LIST

NO.	REVISION	DATE
1	AD-01	06/05/2026

**DRAWINGS MUST BE PRINTED
AND/OR REPRODUCED IN COLOR**

DESIGNED	BY
DRAWN	CR/LC
DATE	04/22/2026

PROJECT NUMBER
25023

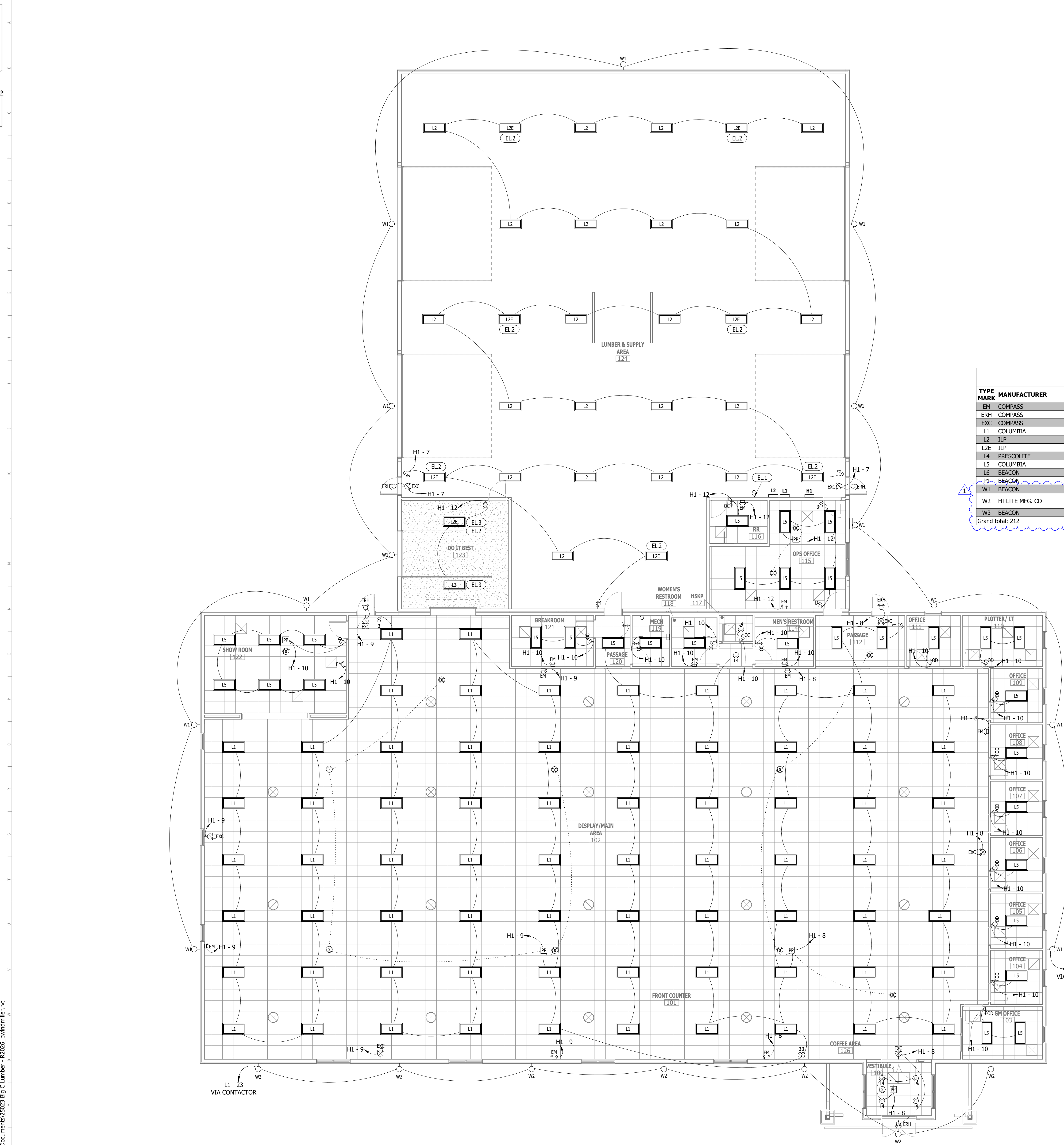
FLOOR PLAN FIRST
FLOOR

A.110

DISCIPLINE
G - GENERAL
C - CIVIL
L - LANDSCAPE
S - STRUCTURAL
A - ARCHITECTURAL
I - INTERIORS
P - PLUMBING
F - FIRE PROTECTION
M - MECHANICAL
E - ELECTRICAL

PHASE
SHEET TYPE
FLOOR LEVEL
SHEET COUNT

A2.220



N
LIGHTING PLAN FIRST FLOOR
1/8" = 1'-0"

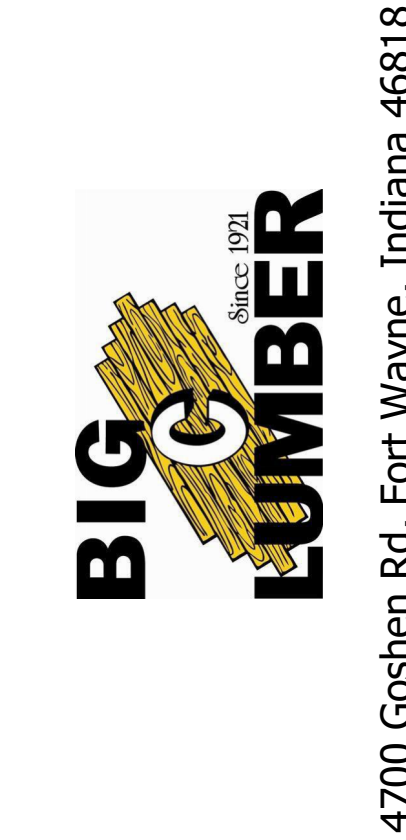
ELECTRICAL KEYNOTES

- EL.1 PROVIDE ASTRONOMICAL TIMER FOR EXTERIOR LIGHTING. MOUNT ON WALL IN UTILITY ROOM. VERIFY LOCATIONS OF CONTACTOR AND LIGHTING TEST SWITCH PRIOR TO ROUGH-IN.
- EL.2 CONNECT SENSE LEAD TO CIRCUIT BEFORE SWITCH.
- EL.3 PROVIDE SURFACE MOUNT KIT.

LIGHTING FIXTURE SCHEDULE										
TYPE MARK	MANUFACTURER	MODEL	LAMP	COLOR TEMPERATURE	LUMENS	WATTAGE	COUNT	TOTAL WATTS	EFFICACY	DESCRIPTION
EM	COMPASS	CL12HLHOSD	LED	4000 K	266 lm	2 W	12	24 W	133 lm/W	LED EMERGENCY LIGHT, BATTERY BACKUP
ERH	COMPASS	CORS	LED	4000 K	133 lm	1 W	5	5 W	133 lm/W	LED EMERGENCY LIGHT REMOTE FIXTURE, POWERED FROM LOCAL EX WITH REMOTE EMERGENCY HEAD CAPABILITY.
EXC	COMPASS	CORRCSO	LED	5000 K	200 lm	4 W	8	32 W	50 lm/W	277V LED EXIT SIGN WITH EMERGENCY LIGHTS
L1	COLUMBIA	LCAT24-40VLG-EDU	LED	4000 K	7491 lm	47 W	70	3304 W	159 lm/W	2X4 LED TROFFER.
L2	ILP	EDV4-12L-U-40-FRL-FIOS	LED	4000 K	12056 lm	84 W	22	1842 W	144 lm/W	HIGH BAY LED FIXTURE, OCCUPANCY SENSOR ON EACH FIXTURE.
L2E	ILP	EDV4-12L-U-40-FRL-FIOS-EM10	LED	4000 K	12056 lm	84 W	8	670 W	144 lm/W	HIGH BAY LED FIXTURE, OCCUPANCY SENSOR ON EACH FIXTURE. BATTERY BACKUP
L4	PRESCOLITE	LBRS1-4RD-M-LSSL-CS9-WH	LED	4000 K	1612 lm	19 W	6	113 W	86 lm/W	4" LED RECESSED DOWNLIGHT, IC RATED HOUSING.
L5	COLUMBIA	LCAT24-40HLG-EDU	LED	4000 K	5877 lm	38 W	31	1163 W	157 lm/W	2X4 LED TROFFER.
L6	BEACON	RFL2-44L-25-4K7-M-UNV-K-DBT	LED	4000 K	3100 lm	25 W	2	50 W	124 lm/W	LED FLOOD LIGHTS FOR SIGN. AIM FOR BEST COVERAGE.
P1	BEACON	RAR1-135-K-3	LED	4000 K	18000 lm	132 W	6	794 W	136 lm/W	25" PARKING LOT LIGHT AND POLE
W1	BEACON	VPW2-80L-45-4K7-4F-UNV-DBT	LED	4000 K	6047 lm	42 W	13	542 W	145 lm/W	LED EXTERIOR WALL LIGHT, MOUNT HIGH ON STRUCTURE +15'
W2	HI LITE MFG. CO	H-15116-91/HL-D-91/20W/LED	LED	4000 K	1800 lm	20 W	7	140 W	90 lm/W	16" DIAMETER WALL-MOUNTED GOOSENECK FIXTURE. BLACK FINISH. COORDINATE LIGHTING FIXTURE LOCATION WITH OWNER'S SIGNAGE ON WALL PRIOR TO ROUGH-IN.
W3	BEACON	VPW2-80L-45-4K7-4F-UNV-DBT	LED	4000 K	6047 lm	42 W	22	917 W	145 lm/W	LED EXTERIOR WALL LIGHT, MOUNT HIGH ON SHED WALL +13'. COORDINATE WITH OWNER.
Grand total: 212									9596 W	



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ELECTRICAL
LIGHTING PLAN

EL.210